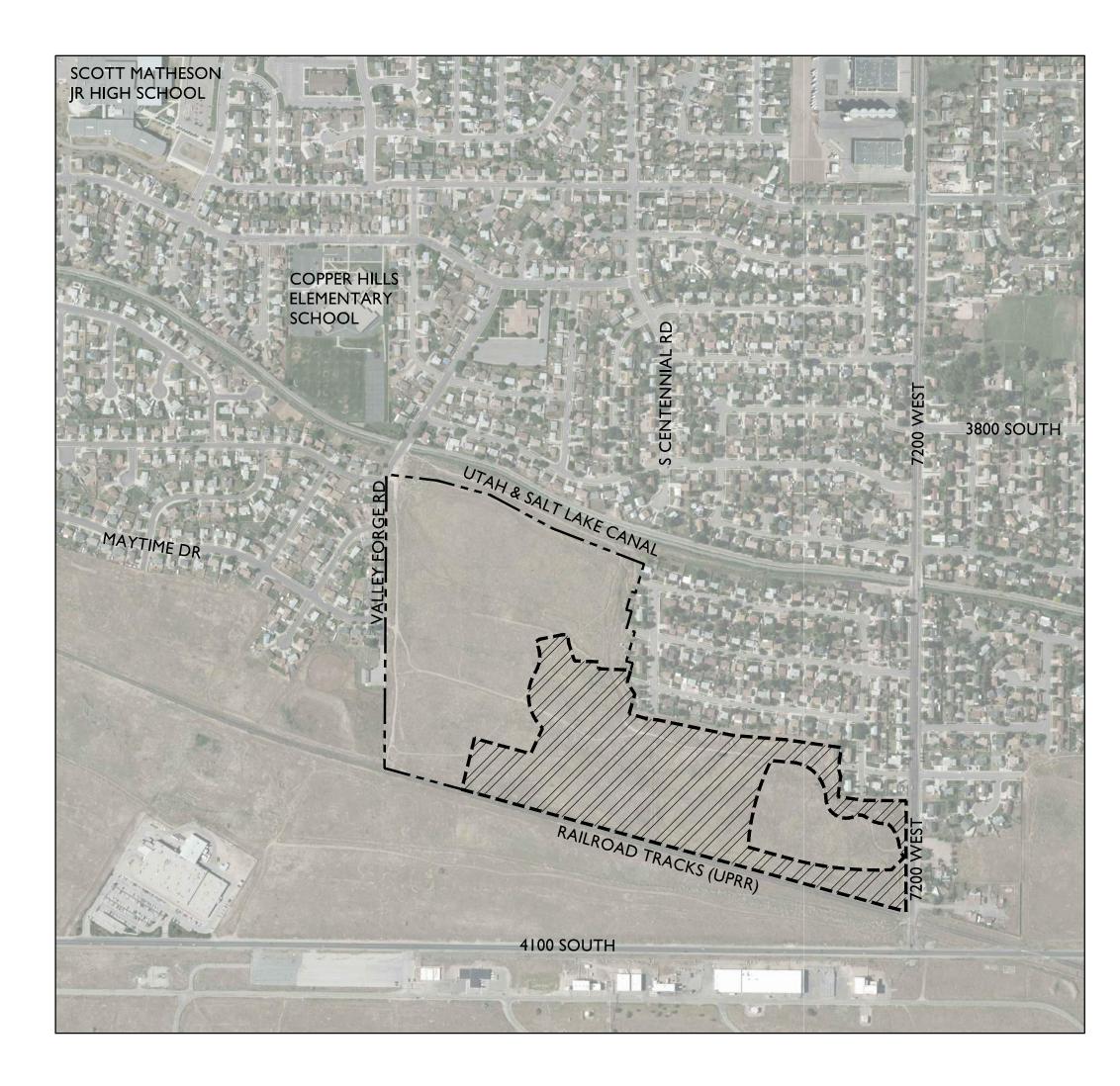
MAGNA REGIONAL PARK - PHASE I

CONSTRUCTION DOCUMENTS - BID SET - VOLUME 1 OF 3



Project Area

4042 South 7200 West Magna, Utah 84044

APPLICABLE CODES

	1
NORT	ł

YEAR

International Fire Code	2015
International Plumbing Code	2014
International Fuel Gas Code	2013
International Building Code	2015
International Mechanical Code	2015
National Electrical Code	2014
International Energy Conservation Code	2015
ADA (ADAAG)	2010
Magna Township Over Pressure Zone Ordinance	Current

G.B:D G. BROWN: DESIGN INC SITE AND LANDSCAPE ARCHITECTS

610 East South Temple, Ste50 Salt Lake City, Utah 84102 p. 801.575.6066 f. 801.575.6166 www.gbrowndesign.com

DESIGN TEAM

Owner

Salt Lake County Parks and Recreation Division 2001 South State Street, Suite S4-700 Salt Lake City, Utah 84114 P: (385) 468-1817 Contact: Dustin Wiberg

Landscape Architect - Prime Consultant

G Brown Design Inc. 610 East South Temple, Suite 50 Salt Lake City, Utah 84102 P: (801) 575-6066 Contact: Andrew Noorlander

Civil Engineer

Perigee Consulting, L.L.C 9089 S. 1300 W., Suite 160 West Jordan, UT 84088 P: (801) 910-3395 F: (801) 590-6611 **Contact: Jed Atherley**

Electrical Engineer

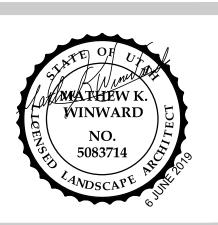
Envision Engineering 240 E. Morris Ave, Suite 200 Salt Lake City, Utah 84115 P: (801) 534-1130 **Contact: Alexsander Rankovic**

Splash Pad

Water Design, Inc. 5047 South Galleria Drive Salt Lake City, Utah 84123 P: (801) 261-4009 **Contact: Brain Anderson**

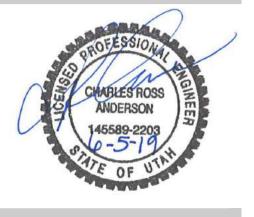
Architect

Range Architecture 1086 E 2100 S Saltk Lake City, Utah 84106 P: (801) 694-9150 Contact: Derek Wilson











VOLUME 1 - SHEET INDEX	•
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Accessible Routes Plan	G-101
Site Prep & Demo Plan	D-100
Site Prep & Demo Details	D-501
Erosion Control Plan	CE-101
Details - Erosion Control	CE-501
Details - Magna Water Standard	CE-502
Residential Street Demolition Plan	C101
Overall Utility Plan	C201
Utility Plan - Water	C202
Utility Plan - Water East Side	C203
Plan & Profile - Sewer Line 1	C301
Plan & Profile - Sewer Line 2	C302
Plan & Profile - 7200 West	C303
Overall Grading	C401
Grading Plan	C402
Grading Plan	C403
Grading Plan	C404
Grading Plan	C405
Overall Playground & Splash Pad Grading	LG-400

Playground & Playground & Playground & Playground & **Overall Layout** Layout Points Layout Points Layout & Mate Playground Su Splashpad & F Splashpad & F Splashpad & F Splashpad & F Layout & Mate Splash Pad De Splash Pad De

Overall Interac Interactive Wa Interactive Wa Interactive Wa Interactive Wat Water Feature **River Feature** Spray Feature Enlarged Spra Equipment Ro **Circulation Eq Circulation Eq** Circulation Equ **Circulation Eq** Splash Pad Se Splash Pad St Splash Pad De Splash Pad De Splash Pad De Splash Pad De **Overall Plantin** Planting Scheo Planting Plan Planting Plan

JUNE 6, 2019

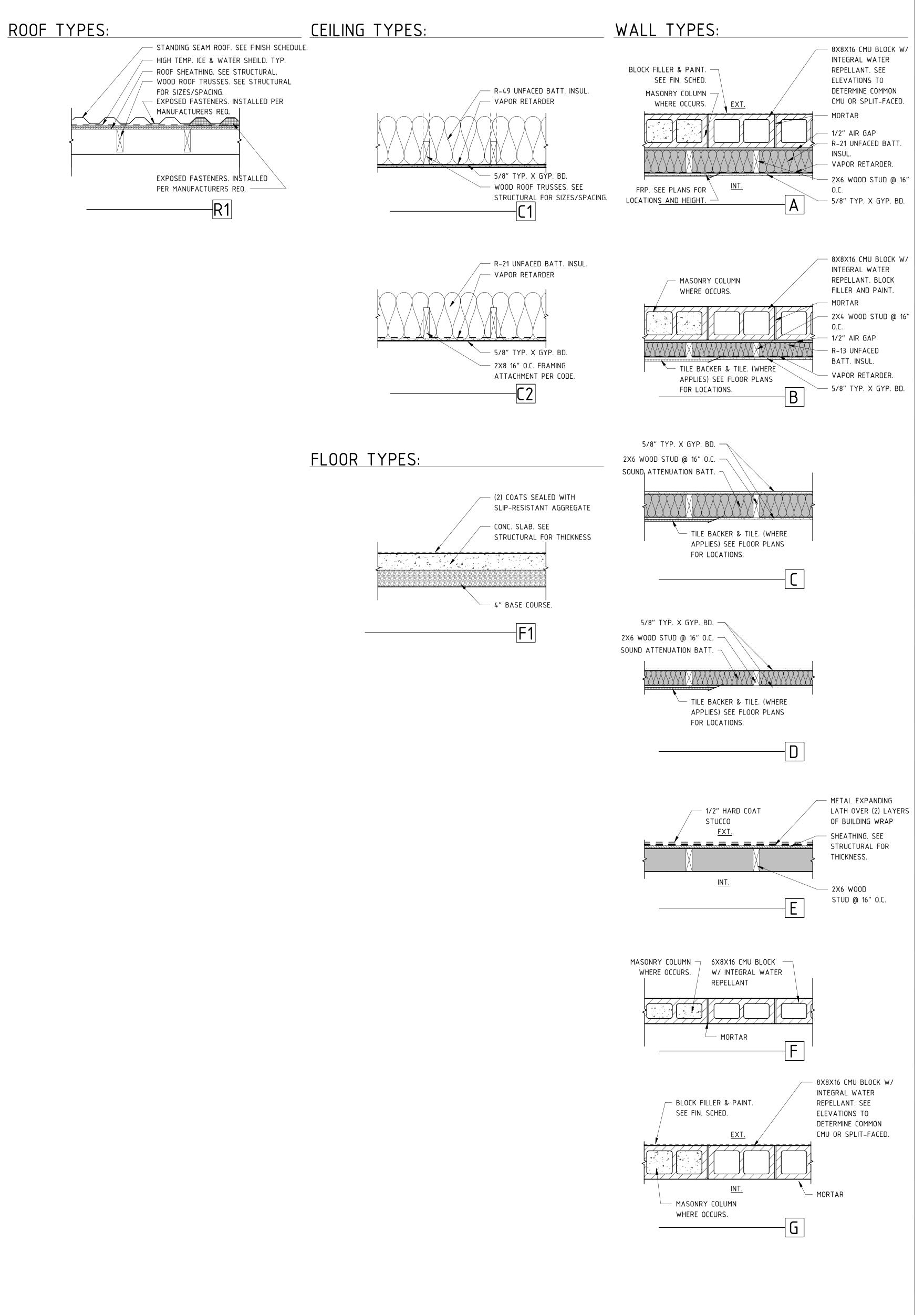
VOLUME 1 - SHEET INDEX

Splash Pad Grading Enlargement	LG-401	Planting Plan	LP-103
Splash Pad Grading Enlargement	LG-402	Planting Plan	LP-104
Splash Pad Grading Enlargement	LG-403	Planting Plan	LP-105
Splash Pad Grading Enlargement	LG-404	Planting Plan	LP-106
It & Materials Plan	LM-001	Planting Plan	LP-107
Schedule	LM-002	Planting Plan	LP-108
Schedule	LM-003	Planting Plan	LP-109
erials Plan	LM-101	Planting Plan	LP-110
erials Plan	LM-102	Planting Details	LP-50 ⁷ LI-00 ⁷
erials Plan	LM-103	Overall Irrigation Plan Irrigation Schedule & Notes	LI-002
erials Plan	LM-104	Irrigation Plan	LI-002
erials Plan	LM-105	Irrigation Plan	LI-102
erials Plan	LM-106	Irrigation Plan	LI-103
erials Plan	LM-107	Irrigation Plan	LI-104
erials Plan	LM-108	Irrigation Plan	LI-105
erials Plan	LM-109	Irrigation Plan	LI-106
erials Plan	LM-110	Irrigation Plan	LI-107
erials Plan	LM-111	Irrigation Plan	LI-108
erials Plan	LM-112	Irrigation Plan	LI-109
urfacing Plan	LM-400	Irrigation Plan	LI-110
Playground Enlargement	LM-401	Irrigation Plan	LI-11
Playground Enlargement	LM-402	Irrigation Details	LI-50 [°]
Playground Enlargement	LM-403	Irrigation Details	LI-502
Playground Enlargement	LM-404		
erials Details	LM-501		
erials Details	LM-502	VOLUME 3 - SHEET	INDEX
erials Details	LM-503	TOEOME O ONEEN	
erials Details	LM-504	Gen. Notes Legends Abbreviations	GI00 ⁻
erials Details	LM-505	ADA Requirements	GI002
erials Details	LM-506	Code Analysis	GI003
erials Details	LM-507	Assembly Types	AE00 ⁻
erials Details	LM-508	Maintenance Building Floor Plans	AE101A
erials Details	LM-509	Equipment/Restroom Building Plans	AE101E
etails	LM-510	Maintenance Building Elevations & Sections	AE201A
etails	LM-511	Equipment/Restroom Building Elevations & Sections	AE201E
		Maintenance Building Wall Sections & Details	AE301A
VOLUME 2 - SHEET	INDEX	Equipment/Restroom Building Sections, Wall Sections Equipment/Restroom Building Wall Dets, Architecture Dets	AE301E AE302E
ativa Watar Eastura Dlan	SD100	Maintenance Building Enlarged Plan and Interior Elevations	AE401A
ctive Water Feature Plan	SP100	Equipment/Restroom Buidlign Enlarged Plan and Int. Elev.	AE401E
ater Feature Plan	SP200	Dr. Sched., Fin. Sched., Acc. Sched., Win. Sched.	AE60 ⁻
ater Feature Equipment Schedule ater Feature Electrical Plan	SP201	Structural General Notes	S0.1
	SP202	Maintenance Building Framing Plan	S1. ⁻
ater Feature Elevation Plan	SP203	Maintenance Building Details	S1.2
e Circulation & Drain Piping Plan	SP300	Restroom Building Framing Plan	S2.
Suction, Supply & Balancing Tank Piping Plan		Restroom Building Details	S2.2 S2.3
es Piping Plan	SP302	Restroom Building Details General Mechanical Notes & Legend	SZ.3 M0.1
ay Features Piping Plan	SP303	Mechanical Plans	M0. M2.1
oom Plan	SP400	Mechanical Schedules & Details	M2. M5.
uipment Schedule	SP401	General Plumbing Notes & Legend	P0.1
uipment Details	SP410	Plumbing Plans	P2.
uipment Details	SP411	Plumbing Schedules and Schematics	P5.
uipment Details	SP412	Plumbing Details	P6.
ections tructural Dataila	SP500	Electrical Symbol Schedules and Sheet Index	EG-100
tructural Details	SP501	Electrical Details	EG-500
etails etaile	SP600	Lighting Risers and Schedules	EG-600
etails	SP601	Electrical Schedules	EG-60 ⁻
etails etaile	SP602	One Line Diagram	EG-602
etails na Dian	SP603	Overall Electrical Site Plan	ES-100
ng Plan	LP-001	Enlarged Electrical Site Plan	ES-10 ⁷
dule & Notes	LP-002	Enlarged Electrical Site Plan	ES-102
	LP-101	Enlarged Electrical Plans	ES-400
	LP-102	Enlarged Electrical Plans	ES-40 ⁻



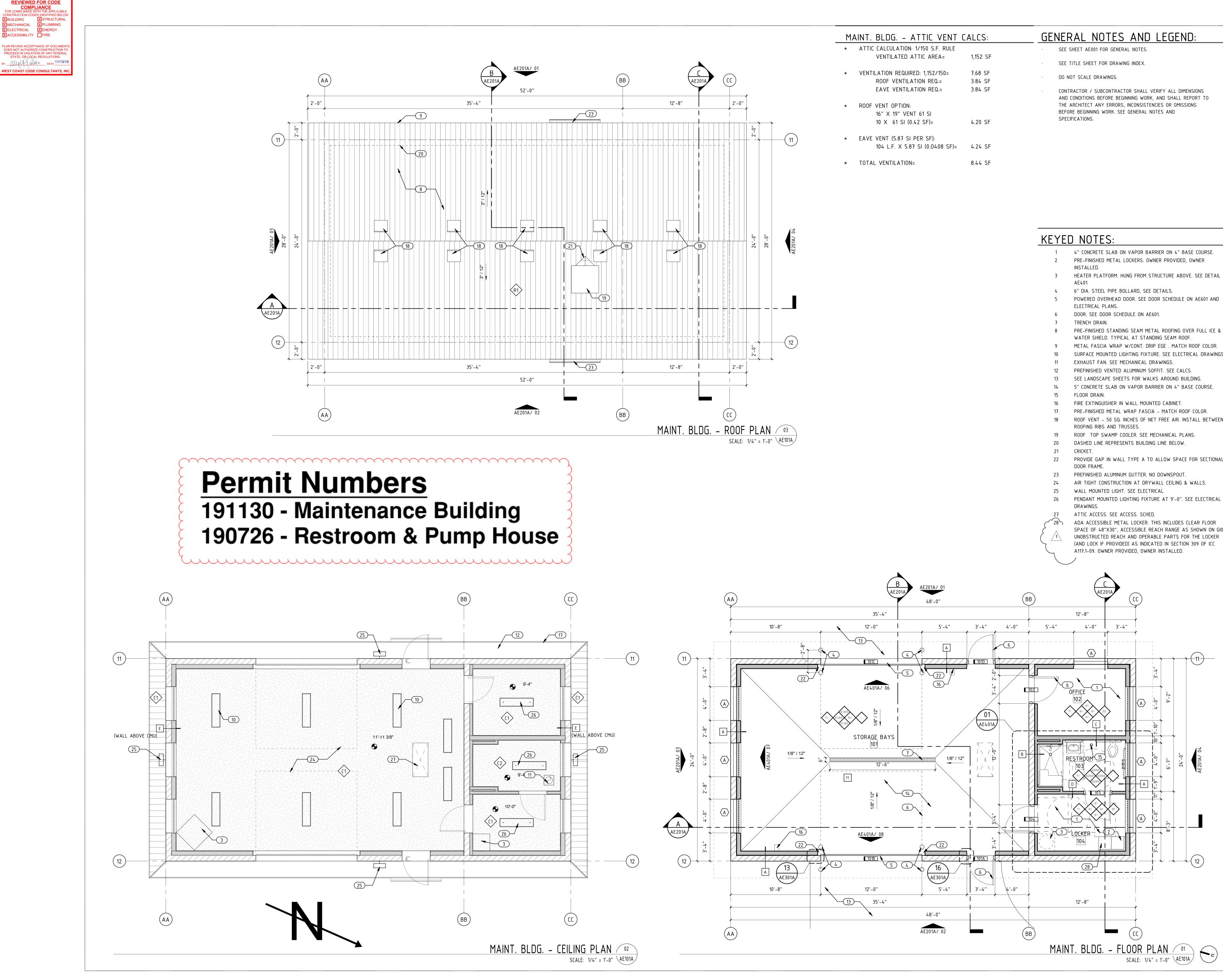
VOLUME 2 - SHEET INDEX





ORMED CONF DOCUMENTS CONSTRUCTION





M	AINT. BLDG. – ATTIC VENT (TALCS:	GENERAL NOTES AND LEGEND:
*	ATTIC CALCULATION: 1/150 S.F. RULE VENTILATED ATTIC AREA=	1,152 SF	 SEE SHEET AE001 FOR GENERAL NOTES. SEE TITLE SHEET FOR DRAWING INDEX.
*	VENTILATION REQUIRED: 1,152/150= ROOF VENTILATION REQ.= EAVE VENTILATION REQ.=	7.68 SF 3.84 SF 3.84 SF	 DO NOT SCALE DRAWINGS. CONTRACTOR / SUBCONTRACTOR SHALL VERIFY ALL DIMENSIONS
*	ROOF VENT OPTION: 16" X 19" VENT 61 SI 10 X 61 SI (0.42 SF)=	4.20 SF	AND CONDITIONS BEFORE BEGINNING WORK, AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
*	EAVE VENT (5.87 SI PER SF): 104 L.F. X 5.87 SI (0.0408 SF)=	4.24 SF	
*	TOTAL VENTILATION=	8.44 SF	

- HEATER PLATFORM. HUNG FROM STRUCTURE ABOVE. SEE DETAIL

- PRE-FINISHED STANDING SEAM METAL ROOFING OVER FULL ICE &
- SURFACE MOUNTED LIGHTING FIXTURE. SEE ELECTRICAL DRAWINGS.

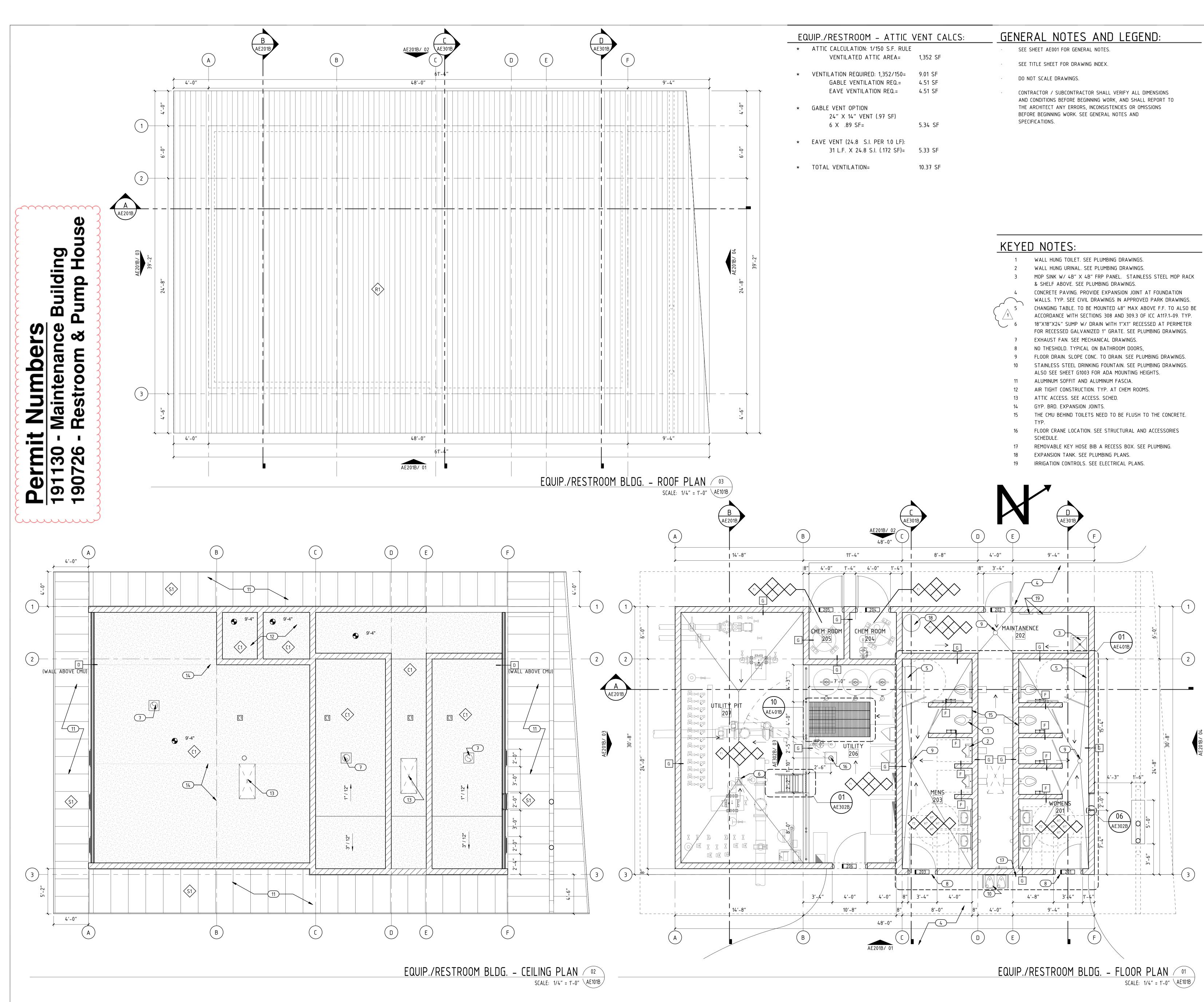
- ROOF VENT 50 SQ. INCHES OF NET FREE AIR. INSTALL BETWEEN

- PROVIDE GAP IN WALL TYPE A TO ALLOW SPACE FOR SECTIONAL

- PENDANT MOUNTED LIGHTING FIXTURE AT 9'-0". SEE ELECTRICAL

ACCESSIBLE REACH RANGE AS SHOWN ON GI002





REVIEWED FOR CODE COMPLIANCE NSTRUCTION CODES IDENTIFIED BE

 Image: State State

PLAN REVIEW ACCEPTANCE OF DOCUMENT DOES NOT AUTHORIZE CONSTRUCTION TC PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.

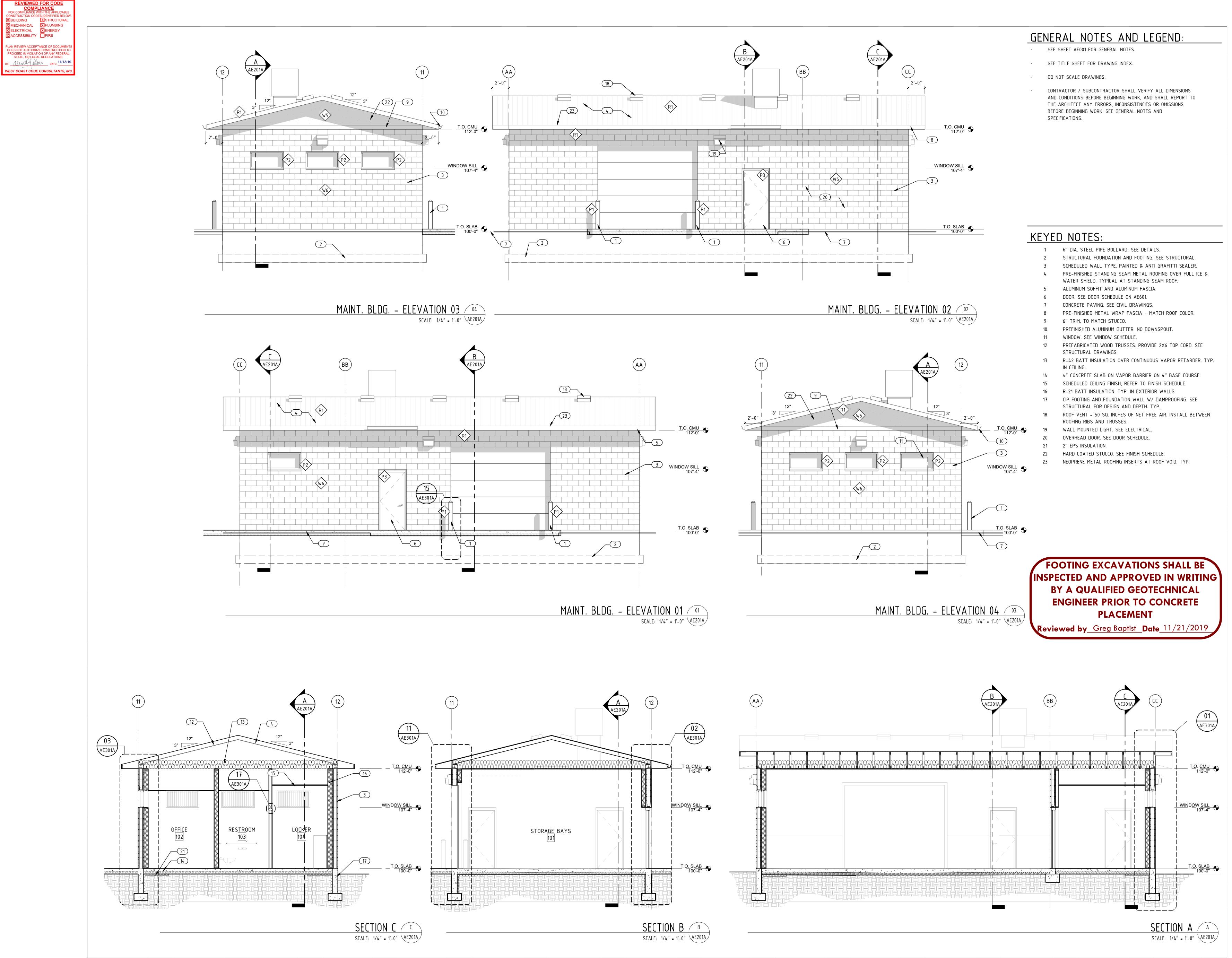
ву: ______ DATE: 11/13/19 WEST COAST CODE CONSULTANTS, IN

XSTRUCTUR

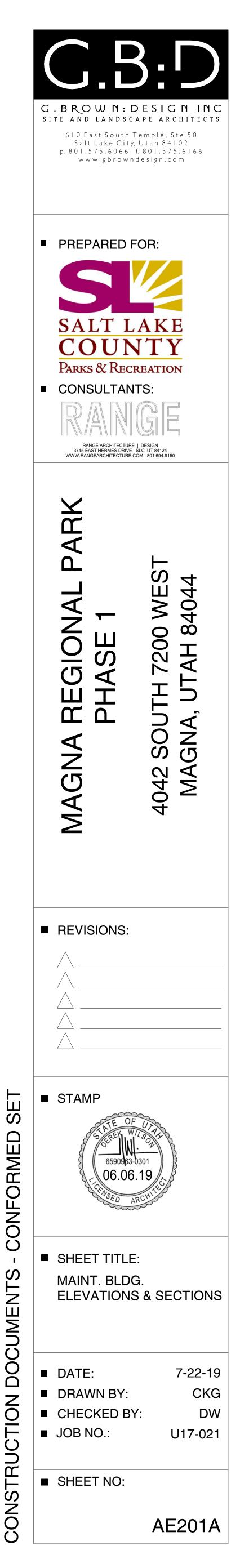
XBUILDING XMECHANICAL XELECTRICAL

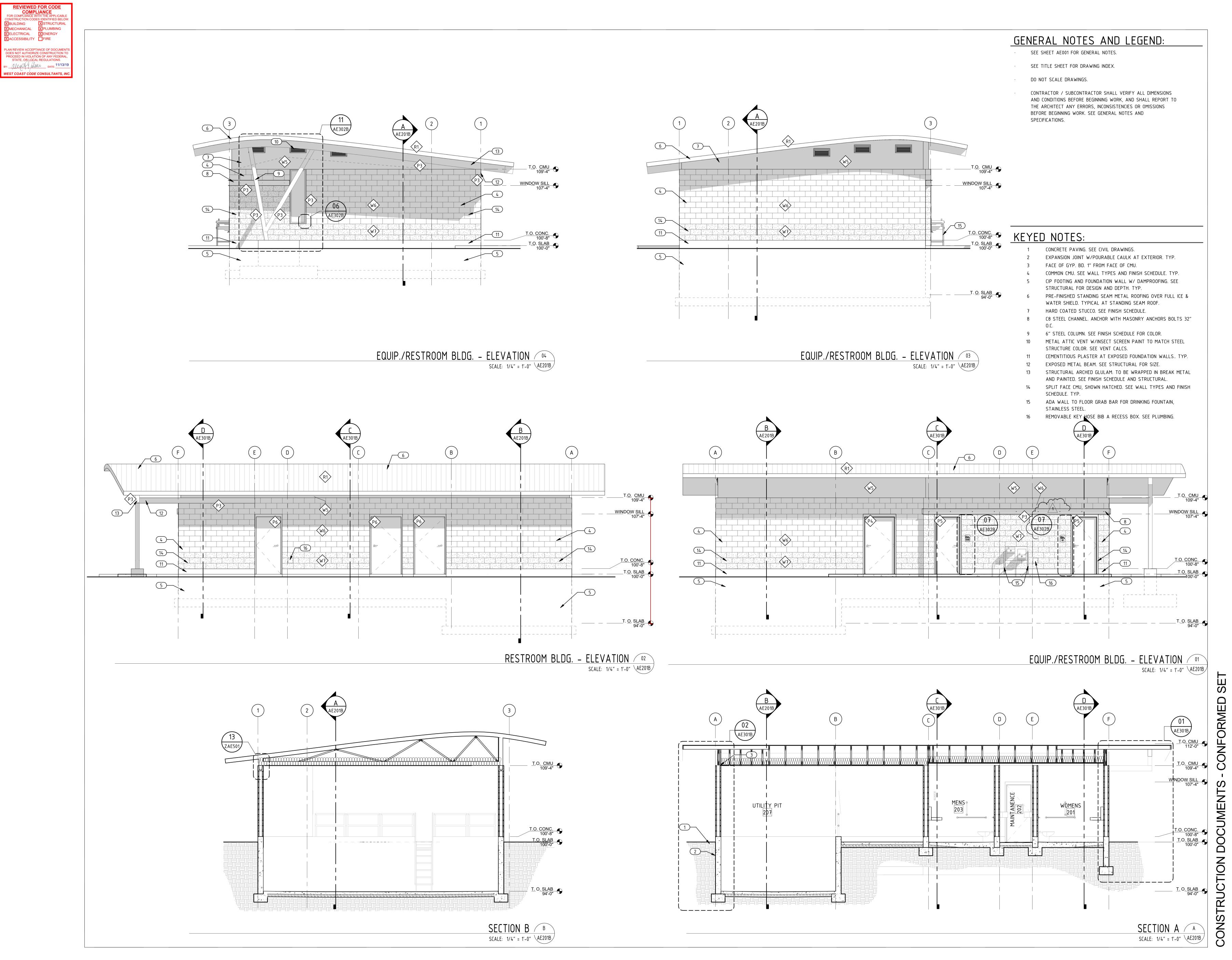
EQUIP./RESTROOM - ATTIC VENT CALCS:	GENERAL NOTES AND LEGEND:
 ATTIC CALCULATION: 1/150 S.F. RULE VENTILATED ATTIC AREA = 1,352 SF 	 SEE SHEET AE001 FOR GENERAL NOTES. SEE TITLE SHEET FOR DRAWING INDEX.
 VENTILATION REQUIRED: 1,352/150= 9.01 SF GABLE VENTILATION REQ.= 4.51 SF EAVE VENTILATION REQ.= 4.51 SF 	 DO NOT SCALE DRAWINGS. CONTRACTOR / SUBCONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK, AND SHALL REPORT TO
* GABLE VENT OPTION 24" X 14" VENT (.97 SF) 6 X .89 SF= 5.34 SF	THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
* EAVE VENT (24.8 S.I. PER 1.0 LF): 31 L.F. X 24.8 S.I. (.172 SF)= 5.33 SF	
* TOTAL VENTILATION= 10.37 SF	

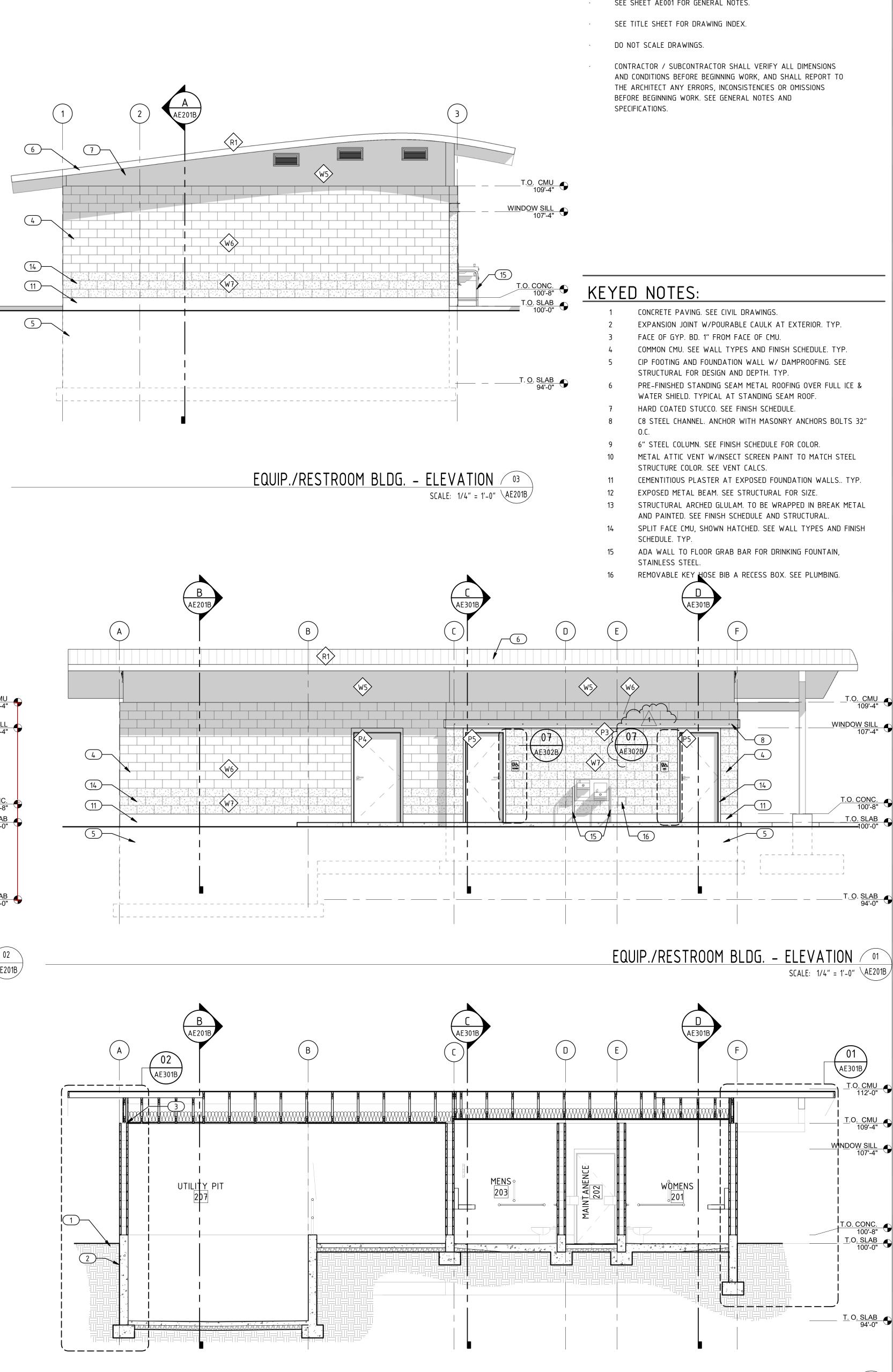




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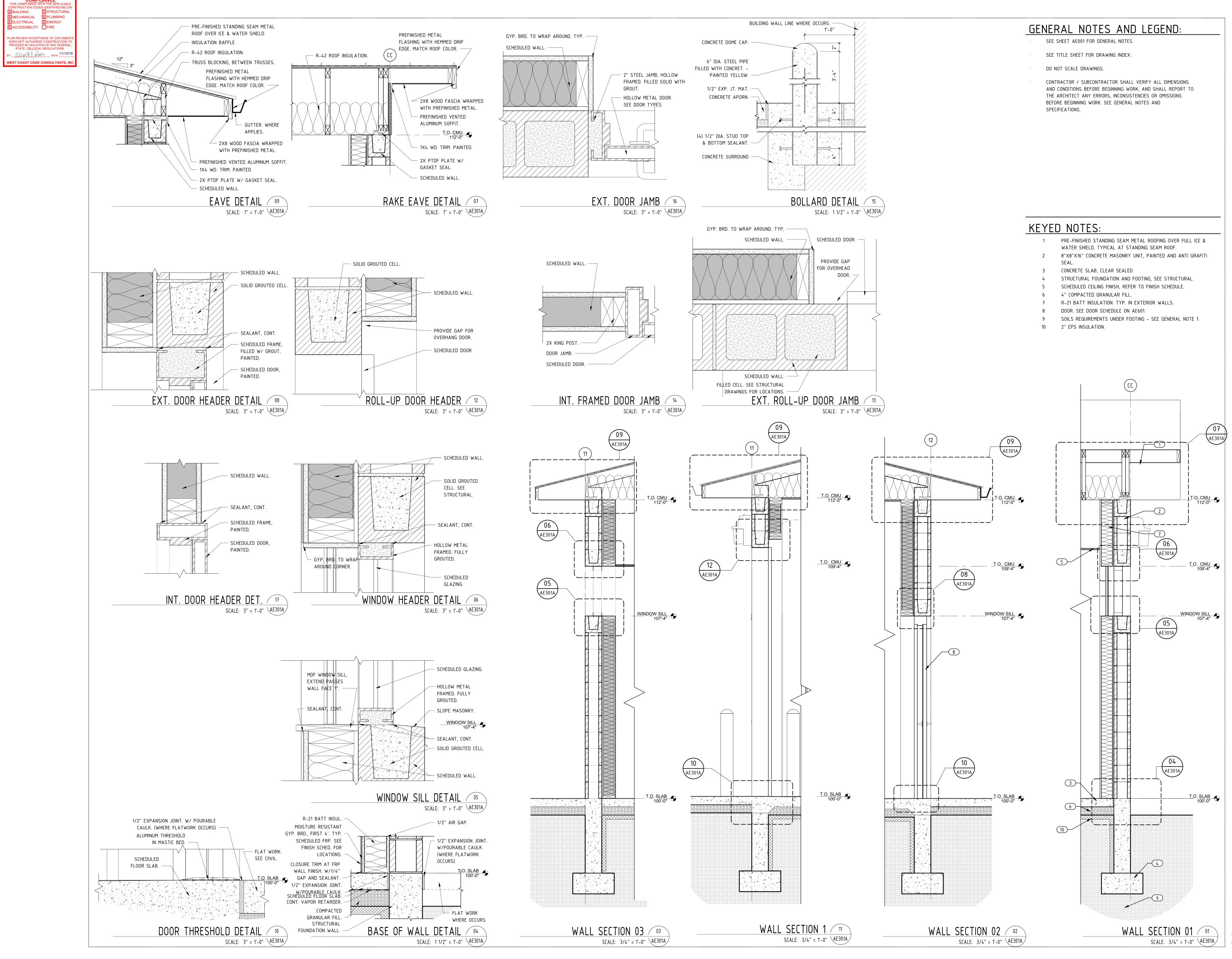






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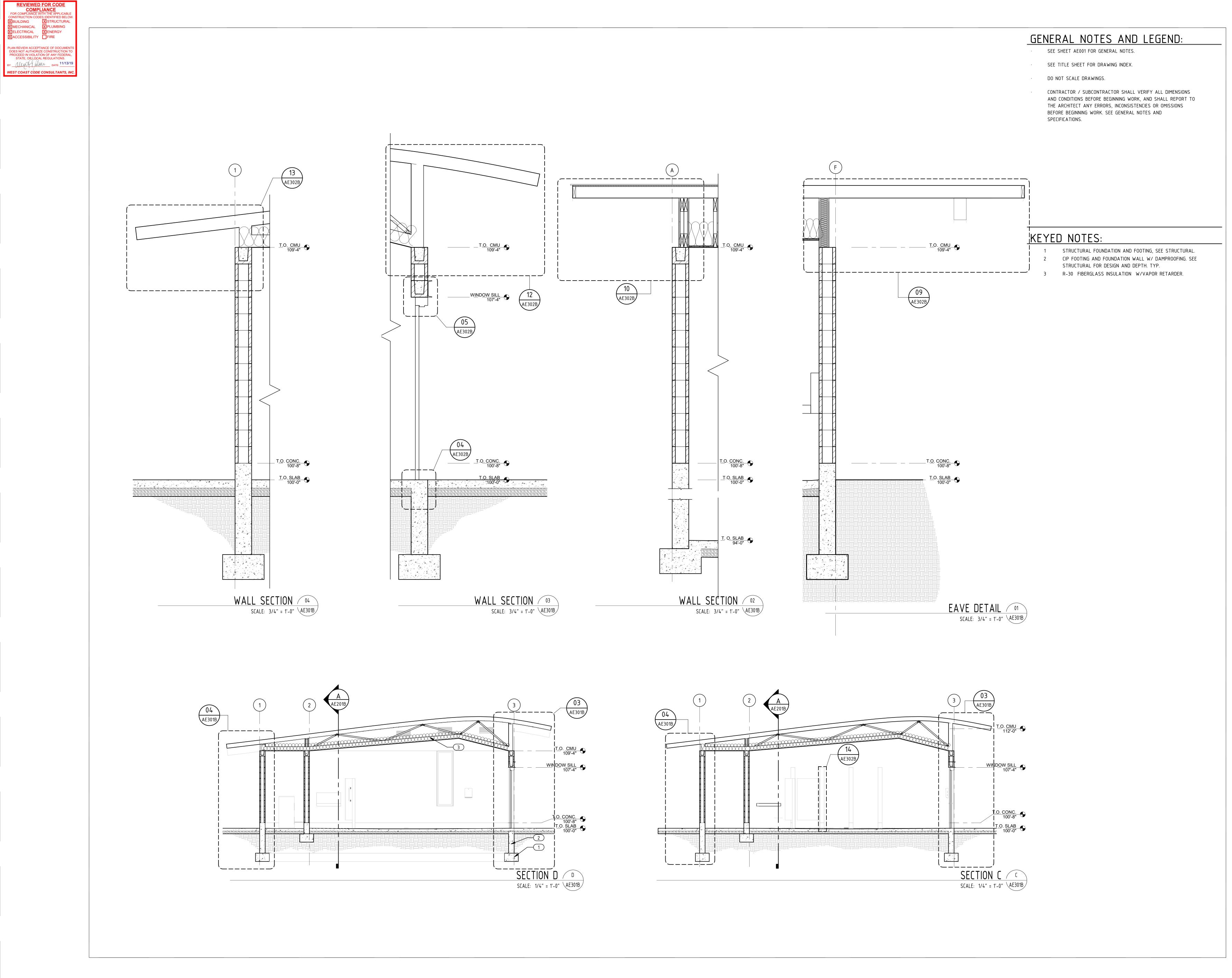


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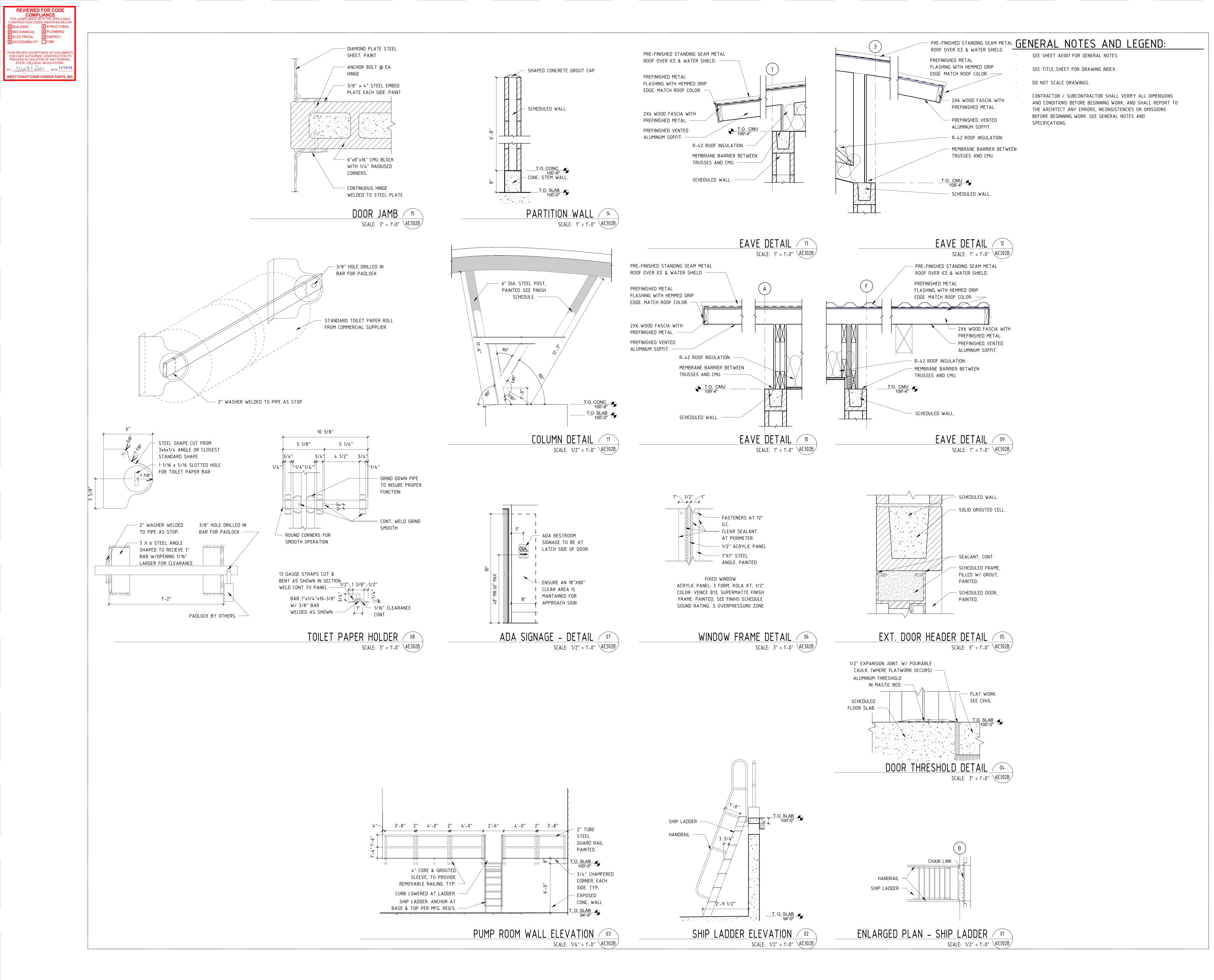
NSTRUCTION C X BUILDING

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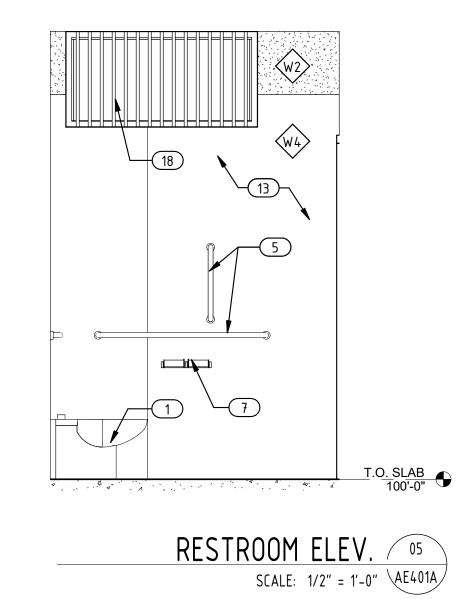


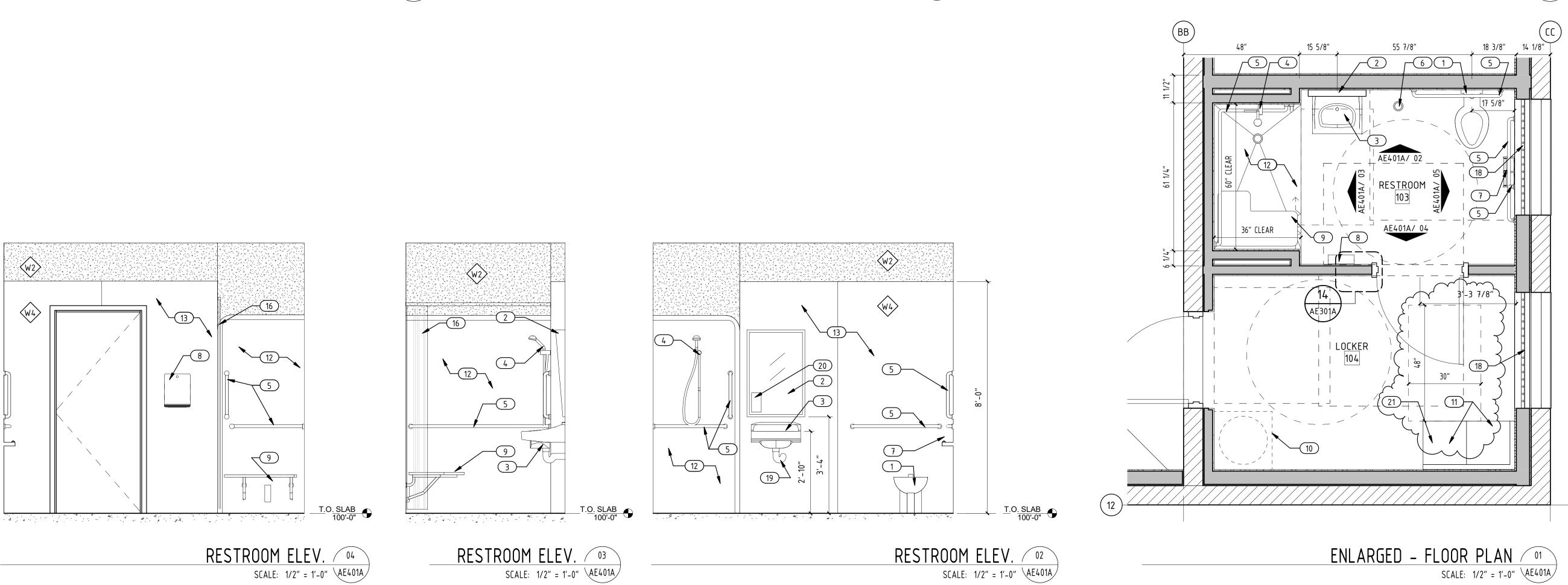


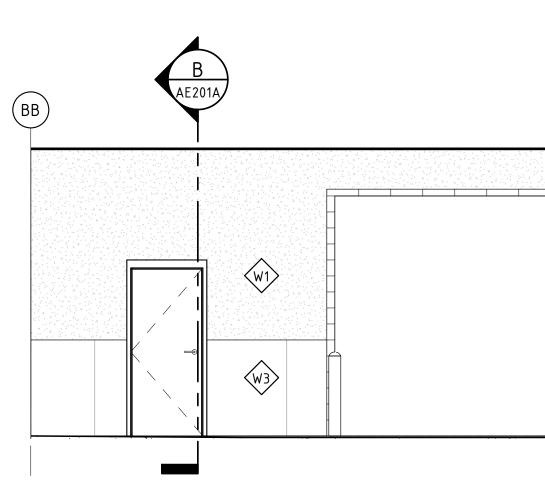












3"X3"X3/16" STEEL ANGLES AT PERIMETER AND INTERMEDIATE BOTH WAYS, WELDED AND PAINTED.

(3) 5/8" DIA. BOLT W/ (2) WASHERS PER WALL ANGLE. (2) 2"X8" WOOD BLOCKING BETWEEN

12 GAUAGE STEEL TRACTION TREAD PLANK FLOORING WITH GALVANIZED FINISH, PAINTED. WELDED TO STEEL

24"

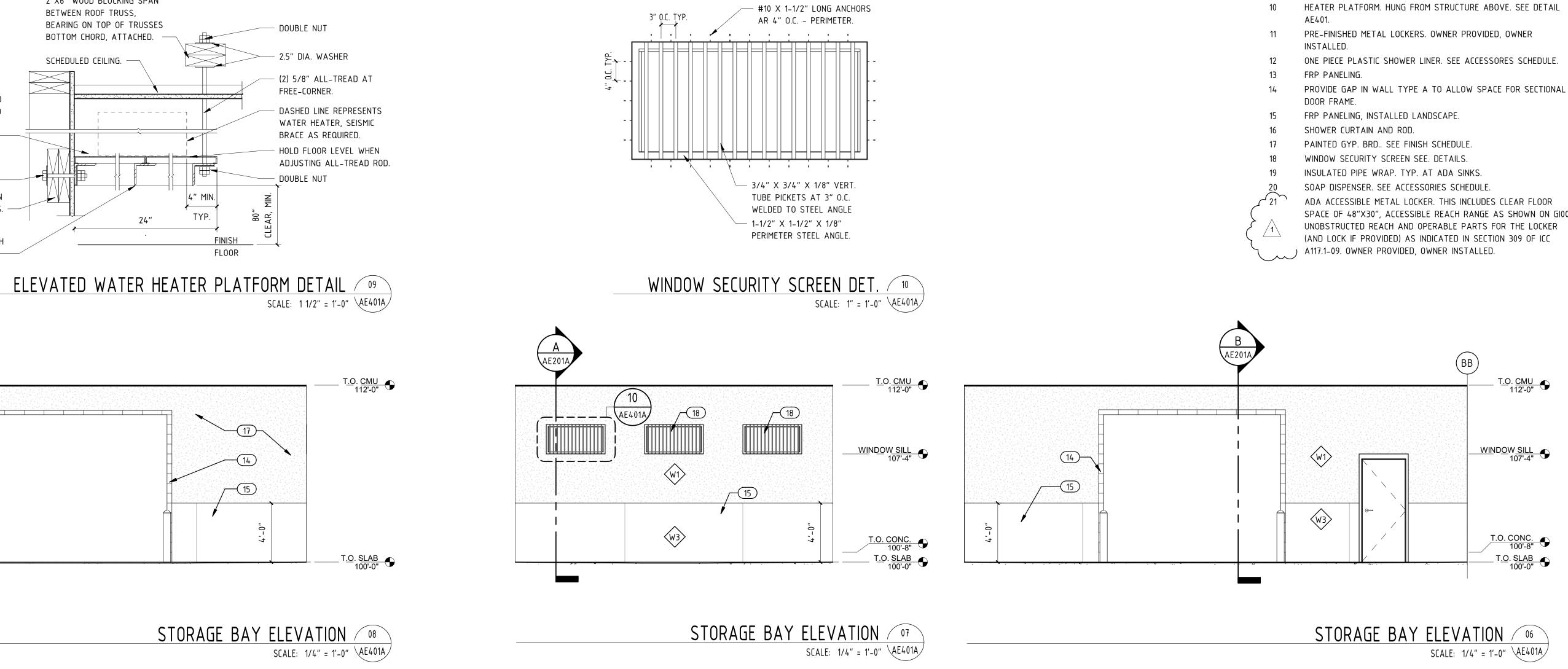
WALL STUDS. ATTACHED TO STUDS.

ANGLES, CONT. -

2"X6" WOOD BLOCKING SPAN BETWEEN ROOF TRUSS, BEARING ON TOP OF TRUSSES BOTTOM CHORD, ATTACHED.

SCHEDULED CEILING.





GENERAL NOTES AND LEGEND:

- SEE SHEET AE001 FOR GENERAL NOTES. SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.

CONTRACTOR / SUBCONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK, AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.

KEYED NOTES:

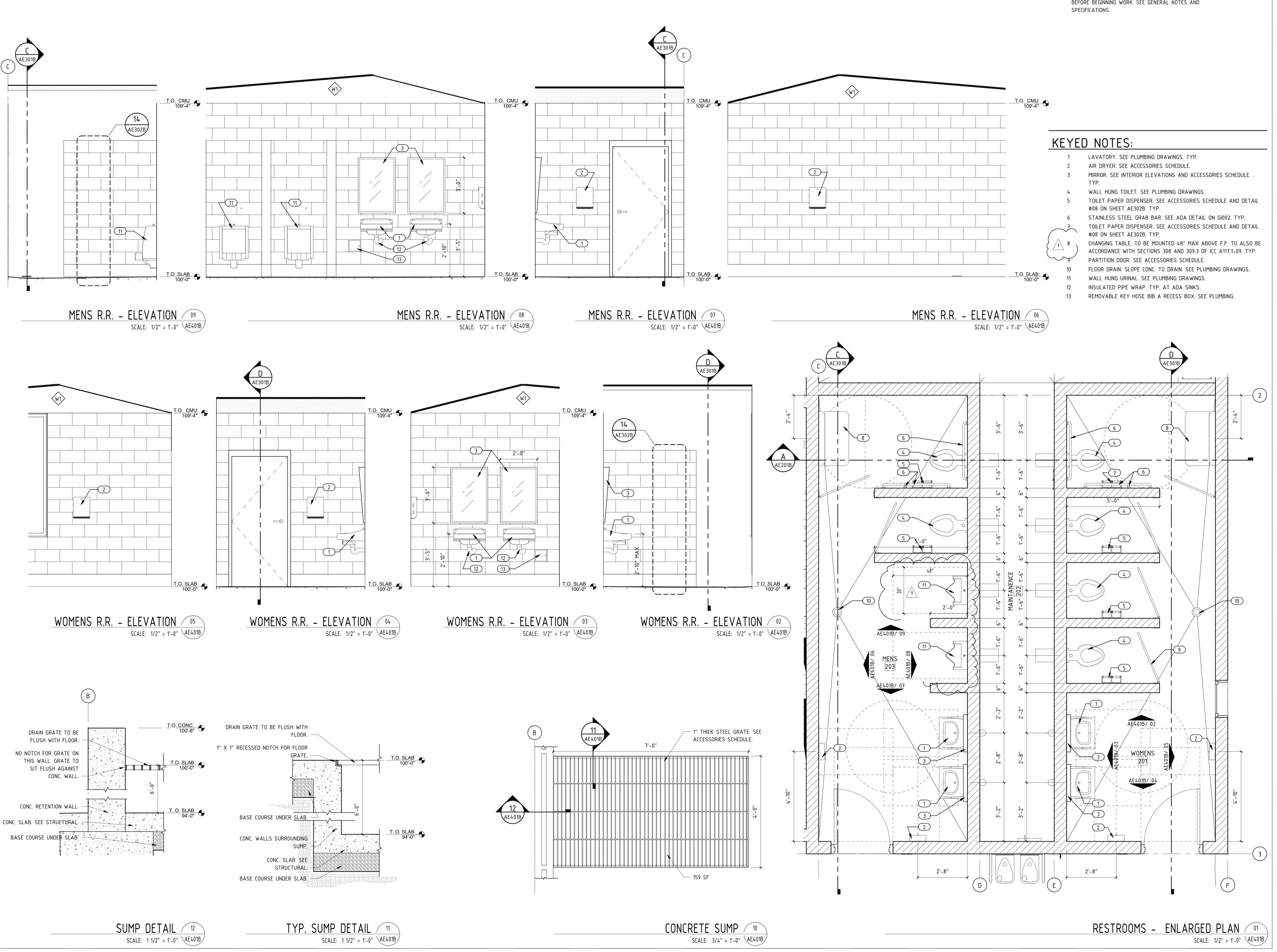
- WALL HUNG TOILET. SEE PLUMBING DRAWINGS. MIRROR. SEE INTERIOR ELEVATIONS AND ACCESSORIES SCHEDULE. 2 TYP.
- LAVATORY. SEE PLUMBING DRAWINGS. TYP.
- ADA COMPLIANT SHOWER WAND, 60" HOSE AND FAUCET CONTROLS. STAINLESS STEEL GRAB BAR. SEE ADA DETAIL ON GIOO2. TYP.
- FLOOR DRAIN. TOILET PAPER DISPENSER. SEE ACCESSORIES SCHEDULE AND DETAIL
- #08 ON SHEET AE302B. TYP. AIR DRYER. SEE ACCESSORIES SCHEDULE.
- FOLD DOWN ADA BENCH. PROVIDE BLOCKING.
- HEATER PLATFORM. HUNG FROM STRUCTURE ABOVE. SEE DETAIL

- SPACE OF 48"X30", ACCESSIBLE REACH RANGE AS SHOWN ON GI002 UNOBSTRUCTED REACH AND OPERABLE PARTS FOR THE LOCKER (AND LOCK IF PROVIDED) AS INDICATED IN SECTION 309 OF ICC









GENERAL NOTES AND LEGEND:

- SEE SHEET AE001 FOR GENERAL NOTES. SEE TITLE SHEET FOR DRAWING INDEX.
- DO NOT SCALE DRAWINGS.
- CONTRACTOR / SUBCONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK, AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND





	DO					
	SIZ	ZE	DNG	DOOR	FRAME	
NO.	WIDTH	HEIGHT	FIRE RATING	ТҮРЕ	ТҮРЕ	HINGES
101A	3'-0"	7'-0"		A	1	01
101B	12'-0''	10'-0"		С	_	_
101C	12'-0''	10'-0''		C	-	_
101D	3'-0"	7'-0"		А	1	01
102	3'-0"	7'-0"		А	1	01
103	3'-0"	7'-0"		А	3	01
104	3'-0"	7'-0"		А	1	01
201	3'-0"	7'-0"		А	1	01
202	3'-0"	7'-0"		А	1	03
203	3'-0"	7'-0"		А	1	01
204	3'-8"	7'-0"		В	2	03
0 0 F	3'-8"	7'-0"		В	2	03
205		7'-0"		А	1	01
205	3'-8"	7-0				
	3'-8"	7-0				

02 CONTINUOUS.

03 NON-REMOVABLE PIN.

04 TAPERED TIPS.

EXIT DEVICE 01 LEVER.

02 PANIC BAR.

LOCKABLE DEVICE

01 PASSAGE02 KEYED CYLINDER.

03 CLASSROOM LOCK.

04 RESTROOM LOCK.

05 STORAGE LOCK.

06 DEADBOLT.07 MORTISE LOCK. (FAIL SAFE)

08 PUSH/PULL.

09 MAGLOCK.

ELECT. STRIKE.
 ELECT. LATCH.

DOOR & FRAME SCHEDULE

HARDWARE

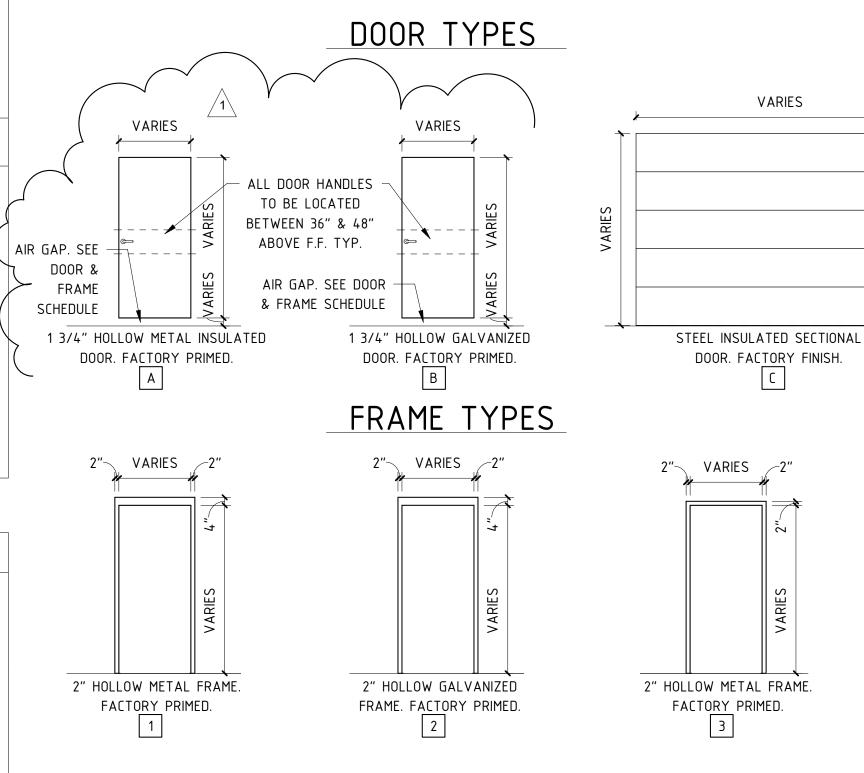
ies	EXIT DEVICES	LOCKABLE DEVICES	KICK PLATE	SILENCERS/ SEALS	ACCESS CONTROL	THRESHOLD	CLOSER	MA
	01/02	02	02	02	_	01/03	01	
	-	_	_	-	-	-	_	
	-	-	_	-	-	-	_	
	01/02	02	02	02	-	01/03	01	
	01	03	-	01	-	-	-	
	01	04	-	01	-	-	_	
	01	04	-	01	-	-	-	
	01	07/09	02	01	04	-	01	
	01	05	02	01	-	-	01	
	01	07/09	02	01	04	-	-	
	01	05	-	01	-	-	-	
	01	05	-	01	-	-	-	
	01	07	02	01	-	-	01	
E REQUIF	L BE LEVER HANDLE, A REMENTS OF IBC 1010 KICK PLATE	ALL DOORS SHALL HAVE (3 .1.9.1. HARDWARE SHALL N)-SILENCERS DOOR IOT REQUIRE PINCH	HARDWARE MFG'S. SEE SPE ING, TIGHT GRASPING, OR T\ MATI	CIFICATIONS. DOOR HARDW WISTING OF THE WRIST TO (ERIAL	VARE SHALL OPERATE.		
	01 EXTERIOR SIZE: HEIGHT MATER 02 INTERIOR			02 03	STAINLESS STEEL POLISHED BRASS OIL RUBBED BRONZE SATIN NICKEL			
	SIZE:	_			BLACK NICKEL			
	HEIGHT MATER				TOM HARDWARE			
	SILENCER/SEALS				N/A			
	01 SILENCER: TYP) .			NG FUNCTION TYPE			
	02 SEAL:				ONE SIDE			
	ACCESS CONTROL				BOTH SIDES			1
	01 CARD READER			-				(
	02 KEYPAD							
	03 PROXIMITY REA	ADER						
	04 EGRESS RELEA	ASE BUTTON						(
	THRESHOLD							
	01 MODEL:			-				
	02 AUTO CLOSE:							
	03 SWEEP.							
	CLOSER							
	01 MODEL:							

	WINDOW AND FRAME SCHEDULE						
	WI	NDOW					
	S	SIZE	TY	PES			
TYPE MARK	WIDTH	HEIGHT	GLAZING	FRAME	SOUND RATING	COMMENTS	
A	4'-0"	2'-0"			.5 OVERPRESSURE ZONE		
В	2'-0"	6'-8"			.5 OVERPRESSURE ZONE		

ACCESSORY SCHEDULE

ACCESSOR	IOUILDOLL							
ITEM	DESCRIPTION	MARK	MATERIAL	MANUFACTURER	COLOR / MODEL #	NOTES:		
		FLOOR						
PARTION DOORS	DIAMOND PLATE STEEL PANEL PARTITION DOOR AND FRAME WITH 4" RADIUSED CORNERS.	F1	CONC. SEALER	-TBD	COLOR: TBD	2 COATS WITH SLIP-RESISTANT AGGREGATE.		
		F2	EPOXY FLOOR COATING	-TBD	COLOR: TBD			
MIRROR	BOBRICK, B-293							
	24" X 36" FIXED/TILT MIRROR	BASE						
TOILET PAPER DISP.	INSTALLED PER DETAILS ON SHEET AE302B	B1	4" RUBBER BASE (NO-TOE)	-TBD	COLOR: TBD			
		WALLS						
HAND DRYER	OWNER PROVIDED, OWNER INSTALLED		GYPBOARD - PAINTED	-TBD	COLOR: GRAY			
		W2	GYPBOARD - EPOXY PAINTED		COLOR: TBD			
HAND SOAP DISP.	OWNER PROVIDED, OWNER INSTALLED		FRP	-TBD	COLOR: TBD	4' WAINSCOT. 4' X 10' INSTALLED LANDSCAPE.		
		W4	FRP	-TBD	COLOR: TBD			
GRAB BARS	2" STAINLESS STEEL		STUCCO	-TBD	COLOR: TBD	HARD COATED STUCCO.		
		W6	EXTERIOR PAINT	-TBD	COLOR: GRAY 1	BLOCK FILLER & PAINT ON COMMON CMU W/ANTI GRAFITTI SEALER.		
		W7	EXTERIOR PAINT	-TBD	COLOR: GRAY 2	BLOCK FILLER & PAINT ON SPLIT-FACE CMU W/ANTI GRAFITTI SEALER.		
CHANGING STATION	GAMCO BCS-2 35" X 4" X 20" COLOR: GRAY	W8	INTERIOR PAINT	-TBD	COLOR: TBD	PAINT W/ANTI GRAFITTI SEALER.		
ATTIC ACCESS LADDER	22"X48" ATTIC LADDER							
		CEILING			1			
		C1	GYPBOARD - PAINTED	-TBD	COLOR: TBD			
SHOWER LINER	PLASTIC SHOWER LINER. MAKE: TBD MODEL: TBD	C2	GYPBOARD - EPOXY PAINTED	-TBD	COLOR: TBD			
METAL LOCKER	18" X18"X5'	ROOFING						
	MAKE: TBD MODEL: TBD	R1	METAL ROOF	MCELROY METAL MEGA RIB	COLOR: PATINA GREEN KYNAR 500 COLOR	PAINT W/ANTI GRAFITTI SEALER.		
FLOOR CRANE	ABELL-HOWE® LIGHT DUTY FLOOR CRANE 4S0033 2000 LB. CAPACITY	S1	METAL SOFFIT	MCELROY METAL MATRIX	COLOR: MATCH ROOF	24 GAUGE PAINT W/ANTI GRAFITTI SEALER.		
			Ĺ		/- II			
		GENERAL			ellow	т		
		P1	EXTERIOR PAINT	-TBD	COLOR: TBD	BOLLARDS.		
		P2	EXTERIOR PAINT	-TBD	COLOR: TBD	WINDOW FRAMES ON MANTIANENCE BLDG.		
		P3	EXTERIOR PAINT	-TBD		COLUMN, GLULAM, STEEL CHANNEL, STEEL BEAM R.R. WINDOW FRAME AND 3- FORM WINDOW (COLOR TO BE CEMENTINE 007. EXACT COLOR AND FINISH TO BE APPROVED BY LANDSCAPE ARCHITECT).		
		P4	EXTERIOR PAINT	-TBD	COLOR: TBD	MAINTENANCE DOORS. SAME PAINT AS W6		
		P5	EXTERIOR PAINT	-TBD	COLOR: TANGERINE (TO MATCH PLAYGROUND	PUBLIC DOORS.		
		P6	EPOXY PAINT	-TBD	COLOR: TBD	CHEM ROOM DOORS.		
		×		R FINISH FINISH, TYP. FINISH				

		KEYING	
	CUSTOM	FUNCTION	
TERIAL	HARDWARE	TYPE	COMMENTS
01	-	01	INSULATED, SEE DOOR HARDWARE
-	-	-	POWERED OVERHEAD, INTERIOR CONTROL BUTTON. SAFETY EYE, INSULATED, PRE-FINISHED.
-	-	-	POWERED OVERHEAD, INTERIOR CONTROL BUTTON. SAFETY EYE, INSULATED, PRE-FINISHED.
01	-	01	INSULATED, SEE DOOR HARDWARE
01	-	01	INSULATED, SEE DOOR HARDWARE
01	-	01	SEE DOOR HARDWARE
01	-	01	INSULATED, SEE DOOR HARDWARE
01	-	01	INSULATED, SEE DOOR HARDWARE
01	-	01	INSULATED, SEE DOOR HARDWARE
01	-	01	INSULATED, SEE DOOR HARDWARE
01	-	01	INSULATED, SEE DOOR HARDWARE. MIN. 3/4" GAP AT BOTTOM OF DOOR. SEE MECHANICAL.
01	-	01	INSULATED, SEE DOOR HARDWARE. MIN. 3/4" GAP AT BOTTOM OF DOOR. SEE MECHANICAL.
01		01	INSULATED, SEE DOOR HARDWARE



FINISH SCHEDULE AND LEGEND





STRUCTURAL NOT GENERAL GENERAL STRUCTURAL NOTES ARE CONSTRUCTION DOCUMENTS THAT SHALL BE INCLUDED WITH THE STRUCTURAL PLANS AND PROJECT

- SPECIFICATIONS. TYPICAL DETAILS AND SCHEDULES SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN.
- "CONTRACTOR" REFERS TO THE CONTRACTOR OR SUB-CONTRACTOR RESPONSIBLE FOR THE PARTICULAR TRADE REFERRED TO IN THE NOTES. THE "CONTRACTOR" SHALL MEET ALL NOTE REQUIREMENTS AND SHALL INCLUDE THE ASSOCIATED COSTS IN HIS/HER BID. C.E. REFERS TO COMPASS ENGINEERING, LLC
- THE GENERAL CONTRACTOR, PROJECT MANAGER, OR SUPERINTENDENT SHALL COORDINATE THE WORK PERFORMED BY ALL TRADES, AND IS ULTIMATELY RESPONSIBLE FOR COMPLIANCE WITH ALL NOTE AND CODE REQUIREMENTS.
- THE CONTRACTOR SHALL PERFORM HIS/HER TRADE AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2015 INTERNATIONAL BUILDING CODE (IBC), AND/OR THE LATEST CODE AND ORDINANCES ADOPTED BY THE LOCAL BUILDING OFFICIAL .
- CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN AND ADJACENT TO THE JOB SITE
- THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND / OR ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BÉTWEEN THE various elements of the working drawings, specifications, and / OR THE NOTES BEFORE PROCEEDING WITH THE FABRICATION OR CONSTRUCTION OF ANY EFFECTED ELEMENTS. ANY WORK DONE BY THE ROOF DEAD LOAD CONTRACTOR BEFORE RECEIVING THE ENGINEERS WRITTEN APPROVAL WILL BE AT THE CONTRACTOR'S RISK/EXPENSE. IN CASE OF CONFLICT, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE PERFORMED AT NO ADDITIONAL COST TO THE OWNER.
- FAILURE TO FOLLOW PLANS AND CONSTRUCTION DOCUMENTS CONSTITUTES CHANGE IN PROJECT SCOPE. THE ENGINEER RESERVES THE RIGHT TO REQUEST REPLACEMENT OF ANY PORTION OF THE STRUCTURE DEVIATING FROM THE PLANS WHERE WRITTEN APPROVAL HAS NOT BEEN OBTAINED. DEVIATION FROM CONSTRUCTION DOCUMENTS WITHOUT WRITTEN APPROVAL RELIEVES ENGINEER OF ALL LIABILITY, AND CONTRACTOR ASSUMES FULL LIABILITY.
- THE CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS, SLOPES 10. AND ELEVATIONS, ETC... (BOTH ON PLANS AND AT THE JOB SITE PRIOR 2. TO DOING WORK), AND SHALL COORDINATE THESE WITH THE ARCHITECT AND ALL TRADES. CONSTRUCTION DRAWINGS SHALL NOT BE SCALED FOR DIMENSIONS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR, PROVIDE AND INSTALLING 11. ALL TEMPORARY SHORING AND BRACING AS NECESSARY. SHORING AND BEAMS SHALL SUPPORT ALL LOADS TO WHICH THE STRUCTURE MAY BI SUBJECTED (i.e. WIND, CONSTRUCTION LOADING, ETC.). SHORING SHALL REMAIN IN PLACE AS LONG AS SAFETY REQUIRES AND/OR UNTIL ALL THE STRUCTURAL ELEMENTS ARE COMPLETED.
- DURING AND AFTER CONSTRUCTION, THE LOADS IMPOSED ON THE STRUCTURE BY THE CONTRACTOR AND OWNER SHALL BE WITHIN THE LIMITS OF THE OCCUPANCY DESIGN LOADS. SEE STRUCTURAL PLANS AND CALCULATIONS FOR THE OCCUPANCY DESIGN LOADINGS AND
- VISITS TO THE JOB SITE BY REPRESENTATIVES OF COMPASS ENGINEERING 5 DO NOT CONSTITUTE APPROVAL OR SPECIAL INSPECTION OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS. STRUCTURAL SHOP DRAWINGS SHALL BE APPROVED BY THE ENGINEER
- AND ARCHITECT OF RECORD PRIOR TO FABRICATION AND ERECTION. Shop drawings shall be stamped by a professional engineer REGISTERED IN THE SAME STATE AS THE PROJECT. SEE STRUCTURAL PLANS AND PROJECT SPECIFICATIONS FOR ADDITIONAL
- STRUCTURAL NOTES AND REQUIREMENTS. ALL COMPONENTS AND SYSTEMS NOT SPECIFICALLY ENGINEERED BY THE ENGINEER OF RECORD SHALL BE "DESIGN—BUILD" BY THE CONTRACTOR. CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SHOP DRAWINGS OR AS-BUILT DRAWINGS STAMPED BY A PROFESSIONAL ENGINEER IF REQUIRED BY THE CITY. IF PRE-ENGINEERED SYSTEM IMPACTS THE ORIGINAL DESIGN FOR INTENT OF THE PROJECT IN ANY WAY, CONTRACTOR SHALL COORDINATE WITH ENGINEER OF RECORD PRIOR TO
- INSTALLATION. PRE-ENGINEERED SYSTEMS SUCH AS JOISTS, TRUSSES, GREENHOUSES, POOLS, DECKS, ETC. SHALL BE ENGINEERED AND DETAILED BY OTHERS UNLESS SPECIFICALLY CONTRACTED OTHERWISE. THE ENGINEER OF - 13. RECORD IS NOT RESPONSIBLE FOR, NOR HAS ANY LIABILITY REGARDING PRE-ENGINEERED SYSTEMS. THE CONTRACTOR SHALL PROVIDE SHOP DRAWINGS AS REQUIRED. JOIST AND TRUSS, ETC.. PROVIDED BY THE ENGINEER ARE FOR COORDINATION AND ESTIMATING ONLY. THE JOIST AND TRUSS MANUFACTURER (OR OTHER MANUFACTURERS) ARE REPSONSIBLE FOR THE ACTUAL DESIGN BASED ON CODE PRESCRIBED, AND ACTUAL LOADS AND FORCES.
- THE ENGINEER OF RECORD IS ONLY RESPONSIBLE FOR ITEMS SPECIFICALLY ENGINEERED BY HIM OR UNDER HIS DIRECT SUPERVISION. 2. CONTRACTOR SHALL COORDINATE WITH MECHANICAL, ELECTRICAL, AND THE ENGINEER OF RECORD IS NOT LIABLE FOR ANY NON-STRUCTURAL ISSUES UNLESS SPECIFICALLY CONTRACTED OTHERWISE. C.E. IS NOT RESPONSIBLE FOR THE COST OF CONSTRUCTION NOR PROJECT BUDGETS, U.N.O. ANY
- STRUCTURAL CHANGES REQUIRED BY THE CONTRACTOR, OWNER, 19. ARCHITECT, ETC.. SHALL BE INVOICED BY C.E. AND TREATED AS ADDITIONAL SERVICES. C.E. SHALL BE COMPENSATED FOR ADDITIONAL ENGINEERING REQUIRED AS A RESULT OF ANY THIRD PARTY OR
- CITY REVIEW. PROVIDED ORIGINAL DESIGN IS IN ACCORDANCE WITH THE CURRENT BUILDING CODE. OMISSIONS IDENTIFIED DURING PLAN REVIEW OR CONSTRUCTION SHALL BE ENGINEERED BY THE ENGINEER OF
- RECORD AT NO ADDITIONAL COST TO THE OWNER. THE OWNER SHALL BE 1. RESPONSIBLE FOR PAYMENT OF OMISSIONS THROUGH AN APPROVED CHANGE ORDER. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION.
- CHECKING OF SUBMITTAL ITEMS BY C.E. IS ONLY FOR GENERAL CONFORMATION WITH THE DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE INFORMATION GIVEN IN THE CONTRACT DOCUMENTS. ANY ACTION SHOWN IS SUBJECT to the requirements of the plans and specifications. CONTRACTOR IS RESPONSIBLE FOR: DIMENSIONS WHICH SHALL BE
- CONFIRMED AND CORRELATED AT THE JOB SITE; FABRICATION PROCESS AND TECHNIQUES OF CONSTRUCTION; COORDINATION OF HIS WORK WITH THAT OF ALL OTHER TRADES AND THE SATISFACTORY PERFORMANCE OF 8. HIS WORK.

<u>JESIG</u>N CODI 2015 INTERNATIONAL BUILDING CODE (IBC)

DESIGN CRITERIA

SLOPED-ROOF SNOW LOAD, PS SNOW EXPOSURE FACTOR, CE SNOW LOAD IMPORTANCE FACTOR, I THERMAL FACTOR, CT BASIC WIND SPEED (3-SEC. GUST) WIND IMPORTANCE FACTOR WIND EXPOSURE INTERNAL PRESSURE

=115MPH

= +/-0.18

1.077G, 0.36G

0.768G, 0.403G

HEARWALLS

=0.384

=40PSF =12PSF

1500 PSF

INCHES

=1.0

- <u>SEISMIC LOADING:</u> SEISMIC IMPORTANCE FACTOR RISK CATEGORY
- BASIC SEISMIC-FORCE-RESISTING SYSTEM = ORDINARY REINFORCED MASONRY

- FLOOR DEAD LOAD EARTHWORK
- <u>Design Criteri</u> Dils Report: DIL BEARING PRESSURE: PROTECTIO COEFF. OF FRICTION
- REQUIREMENTS REQUIKEMENTS CONTRACTOR TO REMOVE ETC. EXISTING FOOTINGS, FOUNDATIONS, SLABS, SITE PAVING, DEBRIS, AND STRUCTURES AS REQUIRED. CONTRACTOR SHALL STRIP THE BUILDING AREA FROM ALL VEGETATION, DEBRIS AND TOPSOIL. CONTRACTOR SHALL EXCAVATE ANY REMAINING LOOSE NATURAL OR FILL SOILS TO EXPOSE COMPETENT NATURAL SOILS. CONTRACTOR SHALL CHECK FOR SOFT SPOTS OR OTHER UNSUITABLE SOILS BY DEOCE POLITING THE ENTIPE BUILDING PAD AREA WITH NORMAL SOILS BY PROOF ROLLING THE ENTIRE BUILDING PAD AREA WITH NORMAL COMPACTION EQUIPMENT. REMOVE UNSUITABLE MATERIALS AND REPLACE WITH COMPACTED ENGINEERED STRUCTURAL FILL OR 2,000 PSI LEAN
- WITH COMPACTED ENGINEERED STRUCTURAL FILL OR 2,000 PSI LEAN CONCRETE. (FLOWABLE FILL). IF THE GROUND WATER IS HIGH, PROOFROLLING IS NOT RECOMMENDED AND 2 FEET OF STRUCTURAL SITE GRADING FULLS ARE RECOMMENDED TO RAISE THE OVERALL SITE GRADE. ENGINEERED OR STRUCTURAL FILL MATERIAL SHALL BE WELL-GRADED, GRANULAR, WITH A MAXIMUM SIZE LESS THAN 4 INCHES, AND NOT MORE THAN 18 PERCENT FINES PASSING A NO. 200 SIEVE. PLACE STRUCTURAL FILL IN MAXIMUM LIFTS OF 8 INCHES. COMPACT STRUCTURAL FILL TO 95 PERCENT OF THE MAXIMUM LABORATORY DENSITY AS DETERMINED BY ASTI D. 1557 JUNO TEST ALL STRUCTURAL FILL FILL MATERIAL AND
- D 1557, UNO. TEST ALL STRUCTURAL FILL. FILL MATERIAL AND PLACEMENT OF ALL FILL MATERIAL MUST MEET THE APPROVAL OF THE ENGINEER SEE PLANS FOR THICKNESS OF ALL FLOOR SLABS. UNDERLAY ALL SI WITH AT LEAST A 4 INCH THICK LAYER OF FREE-DRAINING GRANULAR
- . GRANULAR MATERIAL SHALL BE "PEA" GRAVEL OR ¾ 1 INCI EAN GAP-GRADED GRAVEL. SPECIFICATIONS AND SOILS REPORT FOR FURTHI e projec ARTHWORK REQUIREMENTS. ANY UNFORSEEN CONDITIONS ENCOUNTERED DURING SITE PREPARATION SHALL BE BROUGHT TO THE ATTENTION OF THE SOILS ENGINEER. THE CONTRACTOR SHALL BE RESPONSIBLE TO HAVE ALL SITE SOILS CONDITIONS FIELD VERIFIED.
- EXPANSIVE SOILS, COLLAPSIVE SOILS, SOILS WITH A HIGH LIQUEFACTION POTENTIAL, HIGH WATER TABLE, STEEP SLOPES, ETC. ALL REQUIRE ADDITIONAL ENGINEERING. CONTRACTOR TO COORDINATE WITH PROJECT
- ADDITIONAL ENGINEERING. CONTRACTOR TO COORDINATE WITH PROJECT ENGINEER AND SOILS ENGINEER. IF NO SOILS REPORT HAS BEEN PROVIDED THE SOILS DESIGN CRITERIA HAS BEEN ASSUMED PER TABLE 1804.2 OF THE IBC. A BEARING PRESSURE OF 1500 PSF HAS BEEN USED FOR DESIGN. THE CONTRACTOF AND OWNER ARE RESPONSIBLE TO HAVE ALL SITE CONDITIONS, SOILS, FILLS, ETC.. FIELD VERIFIED PRIOR TO STARTING CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DE-WATERING AS REQUIREE FOR CONSTRUCTION DR CONSTRUCTION.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ANY BRACED, TIEBACK, SLURI WALLS OR SHEET PILING REQUIRED FOR EXCAVATIONS. ALL EARTHWORK, MATERIALS AND PLACEMENT MUST MEET THE APPROVAL OF THE GEOTECHNICAL / SOILS ENGINEER. BACKFILL AROUND FOUNDATION WALLS SHALL BE PERFORMED USING GRANULAR MATERIAL. CARE SHALL BE TAKEN IN PLACING BACKFILL MATERIALS SO AS NOT TO DAMAGE THE FOUNDATION. CONTRACTOR TO

MONITOR AS NEEDED. ONCRETE NOTES

- ALL WORK SHALL BE IN STRICT ACCORDANCE WITH THE 2015 IBC, ACI 31 AND LOCAL ORDINANCES.
- ARCHITECTURAL PRIOR TO PLACING CONCRETE. PROVIDE SLEEVES, BLOCK OUTS, ETC... AS REQUIRED. 3. CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER PLACEMENT OF ALL ANCHOR BOLTS, SEISMIC ANCHORS OR STRAPS, ETC... INSTALL PER MANUFACTURER'S SPECIFICATIONS
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, DETAILING, CARE, PLACEMENT AND REMOVAL OF ALL FORMWORK AND SHORES. DO NOT REMOVE FORMS AND SHORING UNTIL STRUCTURAL MEMBERS ACQUIRE SUFFICIENT STRENGTH TO SUPPORT THEIR OWN WEIGHT PLUS
- CONSTRUCTION LOADS. CONCRETE AND REINFORCING MATERIAL REQUIRED MIN. 28 DAY COMPRESSIVE STRENGTH OF CONCRETE A. FOOTINGS AND FOUNDATIONS: 3000 PSI
- 3. INTERIOR SLABS ON GRADE:

PSI U.N.O.

318-"ACCELERATED CURING".

- CONCRETE OVER STEEL DECK: SITE CONCRETE:
- PROVIDE NORMAL WEIGHT AGGREGATES PER ASTM C-33. U.N.O. PROVIDE TYPE I OR II CEMENT PER ASTM C-150 FOR ALL CONCRETE. MAXIMUM WATER TO CEMENT RATIO IS EQUAL TO 0.50 FOR ALL CONCRIMAXIMUM SLUMP OF CONCRETE IS EQUAL TO 4 INCHES PLUS OR MINU
- INCH. PROVIDE AIR ENTRAINING AS RECOMMENDED BY ACI 318 AND ASTM C-260. DO NOT ADD CALCIUM CHLORIDE TO CONCRETE MIX. THE MAX. CHLORIDE ION CONTENT FOR CORROSION PROTECTION OF REINFORCEMENT IS 0.15% BY WEIGHT OF CEMENT. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL CONCRETE DESIGN REQUIREMENTS.

	1. 2	<u>FOOTINGS</u> ALL FOOTINGS SHALL BE 12" THICK & PROPERLY FORMED. INTERIOR FOOTINGS MAY BE MONOLITHIC WITH SLAB.	S 1.
	2. 3.	ALL EXTERIOR FOOTINGS SHALL BEAR BELOW FROST DEPTH (30 INCHES, FIELD VERIFY) FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL MATERAL, OR ON	
	4. 5.	FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL MATERAL, OR ON PROPERLY PLACED ENGINEERED FILL, SEE EARTHWORK NOTES FOR ADDITIONAL REQUIREMENTS, AND SOILS REPORT. CONTRACTOR SHALL STEP FOOTINGS & FOUNDATION AS REQUIRED.	
	э. 1.	NO FOOTING SHALL BE PLACED IN WATER OR ON FROZEN GROUND. REINFORCEMENT ALL REINFORCING STEEL SHALL BE GRADE 60 BARS PER ASTM A615.	2.
	2. 3.	FIELD BENT DOWELS MAY BE GRADE 40. ALL DEFORMED BAR ANCHORS SHALL CONFORM TO ASTM A496.	3.
	4.	ALL HEADED STUD ANCHORS SHALL CONFORM TO ASTM A108. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM 185. LAP ONE MESH TIE.	4. 5.
	5. 6.	ALL REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH THE ACI DETAILING MANUAL AND ACI STANDARDS (LATEST ADDITION). REINFORCING STEEL AND EMBEDS SHALL BE PROPERLY TIED INTO PLACE PRIOR TO PLACING CONCRETE.	6.
	7.	ALL SPLICES IN REINFORCING BARS SHALL LAP A MINIMUM OF 40 BAR DIAMETERS (U.N.O.). ALL SPLICES SHALL OCCUR IN A COMPRESSION ZONE UNLESS NOTED OTHERWISE. TERMINATE ALL REINFORCING BARS WITH A 90 DEG. BEND OR WITH SEPARATE CORNER BARS. MECHANICAL SPLICES SHALL BE POSITIVE CONNECTING COUPLERS AND CHANICAL SPLICES SHALL BE DESTIVE CONNECTING COUPLERS AND	7.
	8.	MECHANICAL SPLICES SHALL BE POSITIVE CONNECTING COUPLERS AND SHALL MEET ALL APPLICABLE CODE REQUIREMENTS. ADJACENT MECHANICAL SPLICES SHALL BE STAGGERED A MINIMUM OF 24 INCHES ALONG THE REINFORCING BARS. TENSILE CAPACITY OF MECHANICAL SPLICES SHALL BE	8. 9.
	9.	125% OF THE SPLICED BAR. HORIZONTAL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION AND CONTROL JOINTS.	10
	10. 11.	DO NOT SPLICE STIRRUPS AND TIES. DO NOT WELD REINFORCING BARS. DO NOT SUBSTITUTE REINFORCING BARS FOR DEFORMED ANCHOR BARS OR HEADED ANCHOR STUDS.	
	12.	REINFORCEMENT SHALL HAVE THE FOLLOWING CLEAR COVER:	11
	А. a.	CAST-IN-PLACE CONCRETE: i. CAST AGAINST/PERMANENTLY EXPOSED TO EARTH 3" ii. FORMED CONCRETE EXPOSED TO EARTH/WEATHER: #6 THRU #18 BARS 2"	12
	b.	#5 AND SMALLER BARS 1–1/2" iii. CONCRETE NOT EXPOSED EARTH/WEATHER: SLABS, WALLS, JOISTS (#11 AND SMALLER) 3/4"	
ò,	a. b.	SLABS, WALLS, JOISTS (#11 AND SMALLER) 3/4" BEAMS, COLUMNS, TIES, STIRRUPS 1–1/2" FOUNDATION AND RETAINING WALLS	13
	1.	BRACE WALLS AS REQUIRED UNTIL FLOOR SLABS AND/OR FLOOR FRAMING ARE IN PLACE, AND UNTIL WALLS HAVE PROPERLY CURED.	
NL E	2.	BACKFILL ADJACENT TO FOUNDATION WALLS OR IN LANDSCAPED AREAS SHALL BE PLACED IN 8 INCH MAXIMUM LOOSE LIFTS. FILL SHALL BE COMPACTED TO AT LEAST 90% AND HAVE THE MOISTURE CONTENT WITHIN	14
-		2% OF OPTIMUM MAXIMUM DENSITY (ASTM D 1557). HEAVY EQUIPMENT SHALL NOT BE USED TO BACKFILL WITHOUT PRIOR CONSENT OF THE ENGINEER.	1.
- 	3.	SEE ARCHITECTURAL DRAWINGS FOR DRAINAGE METHOD BEHIND FOUNDATION AND RETAINING WALLS.	2. 3.
E RAL 95	4. 5.	CONSTRUCTION JOINTS (COLD JOINTS) IN WALLS SHALL BE WATERPROOFED TO PREVENT LEAKS. DO NOT SPLICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY	4. 5. 6.
95 STM	о. 6.	SHOWN. CONTRACTOR SHALL COORDINATE STEPS IN WALLS WITH THE ARCHITECT, AND SHALL VERIFY WITH COMPASS ENGINEERING.	6. 7.
BS	7.	AND SHALL VERIFY WITH COMPASS ENGINEERING. PROVIDE CORNER BARS AT INTERSECTING WALL CORNERS USING THE SAME BAR SIZE AND SPACING AS THE HORIZONTAL WALL REINFORCING.	8.
CH	8.	PROVIDE VERTICAL DOWELS INTO FOOTINGS AND FOUNDATIONS THAT MATCH THE SIZE AND SPACING OF THE VERTICAL REINFORCEMENT IN THE ABOVE MEMBER.	9.
IER	9.	DO NOT SURCHARGE FDN. AND RETAINING WALLS WITH EQUIPMENT NOR STAGING.	1.
	10. 11.	PROVIDE (2) #5 BARS MIN. AROUND ALL DOOR AND WINDOW OPENINGS. U.N.O. PENETRATIONS THROUGH PANELS SHALL BE REINFORCED BY PROVIDING	2.
		one additional bar at the edge of opening for each bar Interupted by the penetration. Provide Uniform Number of Bars	3.
	12.	EACH SIDE. PROVIDE (2) #5 DIAGONAL BARS ON 4 SIDES TYP. U.N.O. SEE SCHEDULES, TABLES, AND DETAILS FOR ADDITIONAL REINFORCING AND INFORMATION. <u>SLABS</u>	4.
TOR	1.	REINFORCE ALL SLABS W/ #4 @ 18" O.C. EACH WAY, OR WITH 6 x 6 - W2.1xW2.1 WELDED WIRE FABRIC (WWF) UNLESS NOTED OTHERWISE ON	5.
RED		THE PLAN. REINFORCEMENT SHALL BE PLACED 1/4th THE SLAB THICKNESS + 1/2" BELOW THE TOP OF SLAB. WELDED WIRE FABRIC MAY BE 100% VIRGIN POLYPROPYLENE FIBRILLATED FIBERS PER CUBIC	6.
RY	2.	SUBSTITUTED WITH 1.5 POUNDS OF 100 YARD OF CONCRETE. U.N.O. ALL REINFORCING BARS SHALL BE CHAIRED IN THE SLAB. WWF SHALL BE	7.
-	3.	Continuously supported at 36" on center prior to placing Concrete. Begin pour of composite steel deck and concrete floors at or	8.
		NEAR A SUPPORT OR BEARING WALL TO AVOID EXCESSIVE DEFLECTION AND/OR STRESSING OF THE FLOOR STRUCTURE. SEE SUSPENDED SLAB CONSTRUCTION NOTES FOR ADDITIONAL REQUIREMENTS.	9.
0	4. 5.	RECESS FOUNDATION AND POUR SLABS THROUGH, TYPICAL AT ALL EXTERIOR DOORS AND STORE FRONT TYPE WINDOWS.	10
8,		DEPRESS SLABS AS REQUIRED IN AREAS OF CERAMIC TILE, SPECIAL ENTRY MATS, HARDWOOD FLOORS, ETC. COORDINATE LOCATION AND DEPTH WITH THE ARCHITECT.	11 12
	6. 7.	PROVIDE ISOLATION JOINTS AROUND COLUMNS/SPREAD FOOTINGS, AND CONTROL JOINTS AS REQUIRED (I.E., WHERE SLABS TRANSITION IN SIZE). THE CONTRACTOR SHALL ENSURE THAT HEAVY EQUIPMENT AND STAGING	
		REPAIRED OR REPLACED AT NO ADDITIONAL EXPENSE TO THE OWNER.	13
	8. 9.	PROVIDE 2 – #4 BARS X 48 INCHES AT ALL DISCONTINUOUS CONTROL OR CONSTRUCTION JOINTS IN SLAB-ON-GRADE. SPACING BETWEEN CONSTRUCTION OR CONTROL JOINTS IN	
	10.	SLABS—ON—GRADE SHALL NOT EXCEED 15'—O" FOR 4" THICK SLABS AND 20'—O" FOR 5" AND 6" THICK SLABS. THE LENGTH TO WIDTH RATIO OF CONTROL JOINTS SHALL NOT EXCEED 1.25:1. CONSTRUCTION AND CONTROL JOINTS SHALL BE INSTALLED AS	
	11. 12.	DETAILED IN THE DRAWINGS. SAWCUT JOINTS SHALL BE MADE WITHIN 12 HOURS AT PLACING CONCRETE. PROVIDE (1) DIAGONAL #4 BAR x 48" AT ALL INSIDE CORNERS.	M
	12. 13. 14.	ALL SLABS SHALL BE PROPERLY CURED. REFER TO THE ARCHITECTURAL PLANS FOR SPECIFICATION OF ALL FLAT WORK.	1. L0
	15.	WORK. PROVIDE 4" MIN. OF FREE-DRAINING GRANULAR MATERIAL, "PEA" GRAVEL OR 3/4" TO 1" MINUS CLEAN GAP-GRADED GRAVEL, UNDER ALL SLABS-ON-GRADE.	1.
ette. Si 1	16.	PROPERLY CURE ALL CONCRETE. ALL CONCRETE (OTHER THAN HIGH—EARLY—STRENGTH) SHALL BE MAINTAINED ABOVE 50 F AND A MOIST CONDITION FOR AT LEAST THE FIRST 7 DAYS AFTER PLACEMENT,	2.
260.		CONDITION FOR AT LEAST THE FIRST 7 DAYS AFTER PLACEMENT, (HIGH—EARLY—STRENGTH CONCRETE TO REMAIN IN A MOIST CONDITION FOR THE FIRST 3 DAYS) EXCEPT WHEN CURED IN ACCORDANCE WITH ACI	z. 3.

PRIOR TO FABRICATION AND ERECTION. SEE ARCHITECTURAL SHEETS FOR DIMENSIONS AND DECK BEARING FI FVATIONS. MESH 4. SEE ARCHITECTURAL FOR ACCESS HATCHES, DRAFT STOPS, ETC. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL FOR ADDITIONAL STEEL MEMBERS (BRACKETS, ANGLES, ETC...) REQUIRED. SUBMIT SHOP DRAWINGS OF ALL STRUCTURAL STEEL, STEEL JOISTS, STEEL DECKING AND MISCELLANEOUS STEEL TO COMPASS ENGINEERING, LLC. FOR APPROVAL PRIOR TO FABRICATION. ALL STEEL SHALL BE PROPERLY PRIMED EXCEPT AREAS THAT REQUIRE FIELD WELDING. PROVIDE A STANDARD AISC FRAMED CONNECTION FOR ONE HALF THE BEAM'S TOTAL UNIFORM LOAD CAPACITY WHERE A CONNECTION IS NOT SHOWN. STEEL DETAILER SHALL PROVIDE STANDARD STAIR DETAILING INCORPORATING C12 x 20.7 STRINGERS OR APPROVED EQUAL (U.N.O.). SUBMIT DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. 10. PROVIDE ADDITIONAL STEEL AS REQUIRED FOR; POUR STOPS, DECK ANGLES @ ROOF AND FLOORS, DECK SUPPORT ANGLES AS NEEDED, ROOF AND FLOOR DIAPHRAGM CHORDS, CLIP ANGLES, ETC.. AS NEEDED 1. REINFORCE DECK OPENINGS FOR SKYLIGHTS, ACCESS HATCHES, MECHANICAL 3. EQUIPMENT, ETC. WITH L4x4x3/8" OR L6x4x5/16" U.N.O., ON ALL EDGES. ANGLES SHALL SPAN BETWEEN JOISTS AND BETWEEN OTHER ANGLES ETC. AS REQUIRED. USE 1/4" MIN. FILLET WELDS 2. ANY CONNECTION NOT DETAILED SHALL BE THE RESPONSIBLILITY OF THE STEEL FABRICATOR. CONNECTIONS MUST BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER. CONNECTIONS MUST ACCOUNT FOR ALL LOADS & STRESSES INCLUDING BUT NOT LIMITED TO ; GRAVITY, SEISMIC, WIND, THERMAL STRESSES. EXPANSION / CONTRACTION ETC.. 3. CAMBERING OF STEEL BEAMS SHALL BE PROVIDED BY LOCAL STEEL FABRICATOR OF STEEL MILL. SHOP CAMBERING OF BEAMS SHALL BE DONE BY A HEAT/SHRINK METHOD. ANY OTHER METHOD OF CAMBERING SHALL BE APPROVED BY AISC AND PROJECT ENGINEER. 14. ALL EXPOSED STEEL SHALL HAVE WELDS GROUND SMOOTH. MATERIALS WIDE FLANGE SECTIONS: ASTM A992 (50 KSI). OTHER SHAPES AND PLATES: ASTM A36. TUBULAR COLUMNS: ASTM A500 GRADE B (46 KSI). 1. Roofed PIPE COLUMNS: ASTM A501 (36 KSI) OR A53 GRADE B. DEFORMED BAR ANCHORS: ASTM A496 HEADED STUD ANCHORS: ASTM A108 ASTM A307 WITH ASTM A563 HEAVY HEX NUTS ANCHOR BOLTS: WITH HARDENED WASHERS GRADE A (U.N.O.) SAME g BOLTED CONNECTIONS: ASTM A325-N (3/4" DIAMETER MIN.) WELDS: E70 XX AT ALL JOISTS E60 XX ÀT ALL DECKS E70 XX AT ALL OTHER LOCATIONS ALL WELDS AND BOLTING TO MEET APPROVAL OF SPECIAL INSPECTOR AS REQUIRED BY BUILDING OFFICIAL. ALL WELDING AND CUTTING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS ALL INTERSECTING STEEL SHAPES WHICH ARE NOT BOLTED SHALL BE CONNECTED BY A FILLET WELD ALL AROUND, UNLESS NOTED OTHERWISE. NGAND ⊿ FOR THICKNESSES 1/4" AND LARGER. WELD SIZES SHALL BE 1/16" LESS THAN THE THINNEST CONNECTED PART, UNLESS NOTED OTHERWISE. FOR HICKNESSES LESS THAN 1/4", WELD SIZE SHALL BE THE SAME SIZE AS THE THINNEST CONNECTED PART, UNLESS NOTED OTHERWISE. DO NOT WELD REBAR OR ANCHOR BOLTS, INCLUDING "TACK" WELDS. WELD HEADED STUD ANCHORS AND DEFORMED BAR ANCHORS PER MANUFACUTER'S SPECIFICATIONS. TIGHTEN BOLTS BY THE TURN OF THE NUT, CALIBRATED WRENCH, OR DIRECT 2. TENSION INDICATOR METHOD. USE HARDENED WASHERS BENEATH THE TURNED ELEMENT OF ALL BOLTS OR NUTS. ALSO USE HARDENED BEVELED WASHERS TO COMPENSATE FOR THE 7 LACK OF PARALLELISM. PROVIDE HARDENED WASHERS BENEATH THE HEAD AND NUT WHERE A490 BOLTS ARE SPECIFIED PER AISC REQUIREMENTS.). HARDENED WASHERS AND PLATES AT OVERSIZED HOLES SHALL CONFORM TO $_{4.}$ ASTMF-436 AND SHALL .COMPLETELY COVER THE SLOT AFTER INSTALLATION. I. DO NOT REUSE BOLTS, NUTS OR WASHERS. PROVIDE FULL-DEPTH STIFFENER PLATES AT EACH SIDE OF ALL BEAMS AT ALL BEARING POINTS. STIFFENER PLATE THICKNESS EQUALS THE BEAM WEB THICKNESS (1/4" MIN.). FILLET WELD BOTH SIDES OF STIFFENER, ALL AROUND. STANDARD PENETRATIONS THROUGH STRUCTURAL MEMBERS FOR MECHANICAL PLUMBING, ELECTRICAL SYSTEMS, ETC. SHALL BE PROVIDED ON THE CENTER 1 LINE OF THE MEMBER'S DEPTH AND WITHIN THE MIDDLE ONE-THIRD OF HE SPAN. PENETRATIONS LARGER THAN STANDARD (OR GREATER THAN 1/3 THE BEAM DEPTH) ARE NOT PERMITTED WITHOUT PRIOR WRITTEN PROVAL FROM COMPASS ENGINEERING, LLC

ALL WORK TO BE IN STRICT ACCORDANCE WITH THE 2015 IBC, LOCAL

OF STRUTURAL STEEL FOR BUILDINGS" WITH "COMMENTARY", "CODE OF

A325 OR A490 BOLTS", AND "SEISMIC PROVISION FOR STRUCTURAL

ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED BY THE CONTRACTOR

BUII DINGS"

ORDINANCES, AWS STRUCTURAL WELDING CODE, AND THE FOLLOWING AISC 2.

PUBLICATIONS: "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION

STANDARD PRACTICE", SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM 3

WOOD FRAMING NOTES:

ALL WORK TO BE IN STRICT ACCORDANCE WITH THE 2015 IBC, NDS, AND LOCAL ORDINANCES ENGINEERED

- -LAMINATED BEAMS FOR SIMPLE SPANS SHALL BE 24F-V4 DF/DF. -LAMINATED BEAMS FOR CONTINUOUS SPANS AND CANTILEVERS SHALL 24F-V8 DF/DF. DO NOT INSTALL GLU-LAMINATED BEAMS UPSIDE
- Laminated veneer lumber and the like shall be installed per MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS. I—JOISTS SHALL BE TJI OR EQUIVALENT, AND SHALL BE INSTALLED PER
- MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS. ENGINEERED LUMBER, WITH THE EXCEPTION OF GLU-LAMINATED LUMBER, SHALL NOT BE USED IN EXTERIOR APPLICATIONS. 5. USE REDWOOD OR PRESSURE TREATED LUMBER FOR ALL WOOD IN CONTACT WITH CONCRETE, MASONRY, OR EARTH (i.e. MUD SILL).

<u>DIMENSIONAL LUMBER</u> DIMENSIONAL LUMBER USED AS STRUCTURAL FRAMING (i.e. JOISTS,	1.	
RAFTERS AND HEADERS) SHALL BE DOUGLAS FIR-LARCH NO 2 OR FOLIAL		
DIMENSIONAL LUMBER USED FOR STUD WALLS SHALL BE STUD GRADE 2x6 UNLESS NOTED OTHERWISE. SPACE AT 16" O.C. MINIMUM, WITH A DOUBLE TOP PLATE. SPLICES IN THE DOUBLE TOP PLATE SHALL ALTERNATE TOP	2.	
AND BOTTOM. ALL SILL PLATES ARE TO BE BOLTED TO FOUNDATION $w/\frac{5}{8}$ " DIA x 10" J-BOLTS @ 32" O.C. MAXIMUM, UNLESS NOTED OTHERWISE ON THE	Ζ.	
STRUCTURAL DRAWINGS AND SHEARWALL SCHEDULE.	3.	
IN NO CASE SHALL 2 X 4" BEARING WALLS SUPPORT MORE THAN TWO FLOORS OF FRAMING IN ADDITION TO ROOF AND CEILING. REFER TO CONSTRUCTION DOCUMENTS FOR ROUGH CUT TIMBER USED AS	4.	
STRUCTURAL FRAMING. ALL NAILS SPECIFIED ON THE DETAILS AND SCHEDULES SHALL BE COMMON	5.	
NAILS UNLESS NOTE OTHERWISE. COLUMNS	5. 6.	
ALE COLUMNS SHALL EXTEND DOWN THROUGH THE STRUCTURE TO THE	7.	
COLUMNS SHALL BE BRACED AT EACH FLOOR LEVEL. POSTS SHALL BE DOUGLAS FIR—LARCH NO. 1 OR EQUAL. BEARING POINTS OF COLUMNS ARE TO BE SUPPORTED BY ADDITIONAL	8.	
BUILT-UP BLOCKING AT JOISTS AND RAFTERS EQUAL TO THE NUMBER OF PLYS IN POST OR EQUAL TO WIDTH OF POST. BLOCKING SHALL BE	9.	
CONSTRUCTED USING RIM BOARD MATERIAL OR SOLID SAWN LUMBER.	10.	
<u>FLOOR, ROOF AND WALL SHEATHING</u> ALL ROOF SHEATHING SHALL BE A MINIMUM OF 5/8" APA EXP. 1 RATED		
ALL ROOF SHEATHING SHALL BE A MINIMUM OF 5/8" APA EXP. 1 RATED SHEATHING OR EQUAL WITH 10d COMMON NAILS AT 6" O.C. PERIMETER, 6" O.C. PANEL EDGES, AND AT 12" O.C. IN THE FIELD UNLESS NOTED OTHERWISE ON CHARGEN DE OCHEDULE.	1.	
PROVIDE ON SHEATHING SCHEDULE. PROVIDE 2 X SHAPED BLOCKING AT RIDGES UNLESS A CONTINUOUS MEMBER EXISTS. PANEL EDGES ARE UNBLOCKED UNLESS NOTED	2. 3.	
OTHERWISE ON THE STRUCTURAL PLANS. ALL FLOOR SHEATHING SHALL BE A MINIMUM OF 3/4" THICK T&G		
SHEATHING GLUED AND NAILED WITH 10d COMMON NAILS OR EQUAL AT 6" O.CPERIMETER, 6"_O.C. PANEL EDGES, AND AT 10"_O.C. IN THE FIELD	4.	
UNLESS NOTED OTHERWISE ON SHEATHING SCHEDULE. PANEL EDGES ARE		
ALL EXTERIOR WALLS SHALL BE SHEATHED WITH 7/16" APA EXP. 1 RATED SHEATHING OR EQUAL WITH 8d COMMON NAILS AT 6" O.C. EDGES AND AT	5.	
12" O.C. IN THE FIELD – FLAT BLOCKED AT ALL PANEL EDGES, UNLESS NOTED OTHERWISE IN THE STRUCTURAL PLANS AND SHEAR WALL	•	
SCHEDULE. AT ROOF AND FLOOR DIAPHRAGMS, PANEL EDGE NAILING IS TO INCLUDE DRAG STRUTS, TENSION CHORDS, BLOCKING, OVER BEARING WALLS AND	6.	
DRAG STRUTS, TENSION CHORDS, BLOCKING OVER BEARING WALLS AND SHEAR WALLS, AND ANY OTHER SPECIAL DIAPHRAGM MEMBERS NOTED ON PLANS.		
AT SHEAR WALLS, PANEL EDGE NAILING IS TO INCLUDE TOP AND BOTTOM PLATES, END POSTS, ALL VERTICAL ELEMENTS @ HOLDOWN ANCHORS, AND HORIZONTAL BLOCKING. ALL PANEL EDGES MUST BE BLOCKED.	1.	
STRUCTURAL CONNECTIONS	2.	
THE CONTRACTOR IS ULTIMATELY RESPONSIBLE TO PROVIDE ADEQUATE STRUCTURAL CONNECTIONS. CONNECTIONS MUST CARRY THE BEARING CAPACITY OF THE MEMBER AND ANY UPLIFT OR SEISMIC FORCES	3.	
GENERATED IN THE MEMBER. SPECIAL CONSIDERATION SHALL BE GIVEN TO PREVENT CRUSHING OF THE MEMBER AT BEARING, SPLITTING AND /	4.	
OR CRACKING OF THE WOOD, ETC. WRITTEN PRIOR APPROVAL FROM COMPASS ENGINEERING IS REQUIRED FOR	4	
ANY DEVIATION FROM THE CONSTRUCTION DOCUMENTS. COMPASS ENGINEERING IS NOT RESPONSIBLE FOR CONNECTIONS NOT APPROVED	1.	
PRIOR TO CONSTRUCTION OR INSTALLATION. PROVIDE SIMPSON CONNECTIONS OR EQUAL IF CONNECTION DETAILS ARE NOT PROVIDED IN THE CONSTRUCTION DOCUMENTS. INSTALL PER	2. 3.	
MANUFACTURERS RECOMMENDATIONS. REQUEST ADDITIONAL ASSISTANCE FROM COMPASS ENGINEERING IF NON—STANDARD CONNECTIONS ARE	0.	
REQUIRED. ALL STRUCTURAL MEMBERS SHALL HAVE 1 3/4" BEARING (MINIMUM). SEE SCHEDULES IN THE 2015 IBC FOR ADDITIONAL NAILING PATTERNS.	4.	
SEE SCHEDULES IN THE 2015 IBC FOR ADDITIONAL NAILING PATTERNS. FASTENERS USED BELOW GRADE IN PONY WALLS, CRIPPLE WALLS OR		
FASTENERS USED BELOW GRADE IN PONY WALLS, CRIPPLE WALLS OR KNEE WALLS AND FASTENERS USED TO ATTACH SHEATHING TO THE EXTERIOR FACE OF EXTERIOR BASEMENT OR CRAWLSPACE WALL STUDS SHALL BE TYPE ADOVE OD ATTACH EXTERIOR TO DECEMPT	5.	
TREATED SILL PLATES SHALL BE OF TYPE 304 OR 316 STAINLESS STEEL,		
SILICON BRONZE, COPPER, HOT—DIPPED GALVANIZED (ZINC COATED) STEEL NAILS, OR HOT—TUMBLED GALVANIZED (ZINC COATED) STEEL NAILS.	6.	
ELECTRO—GALVANIZED STEEL NAILS AND GALVÀNIZED (ZINC COATED) STEEL STAPES SHALL NOT BE PERMITTED.	7.	
BLOCKING, BRIDGING, MISCELLANEOUS. ALL JOISTS AND RAFTERS SHALL HAVE FULL-HEIGHT SOLID BLOCKING AT	_	<u> </u>
THEIR BEARING POINTS. CONNECT EACH BLOCK TO THE TOP OF EXTERIOR WALLS WITH SIMPSON A34 CLIPS (U.N.O.). EACH RAFTER AND/OR ROOF TRUSS SHALL BE ANCHORED WITH SIMPSON H1 ANCHORS AT EACH END. I-JOIST JOISTS USED AS JOISTS AND RAFTERS SHALL HAVE FULL-HEIGHT	1. 2.) T
I-JOIST JOISTS USED AS JOISTS AND RAFTERS SHALL HAVE FULL-HEIGHT SOLID BLOCKING AT THEIR BEARING POINTS. CONNECT EACH BLOCK TO	۷.	S
THE TOP OF EXTERIOR WALLS WITH SIMPSON A34 CLIPS (U.N.O.). EVERY OTHER I-JOIST RAFTER SHALL BE ANCHORED WITH A SIMPSON H3 CLIP.	1.	/ V
INSTALL BRIDGING AT THE MID—SPAN OF ALL FLOOR JOISTS AND/OR AT 8'-0 O.C. (WHICH EVER IS SMALLER). INSTALLATION SHALL BE PER		Å
MANUFACTURER'S RECOMMENDATIONS AND SPECIFICATIONS TO AVOID EXCESSIVE FLOOR VIBRATION AND/OR SQUEAKING. STANDARD PENETRATIONS THROUGH STRUCTURAL MEMBERS FOR		T
MECHANICAL, PLUMBING, ELECTRICAL SYSTEMS, ETC. SHALL BE PROVIDED		3 C T
ON THE CENTER LINE OF THE MEMBER'S DEPTH AND WITHIN THE MIDDLE ONE—THIRD OF THE SPAN. LARGER THAN STANDARD PENETRATIONS ARE NOT PERMITTED WITHOUT PRIOR APPROVAL.	2.	S
BIRDS MOUTHS AND/OR NOTCHING OF STRUCTURAL MEMBERS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL PLANS IS NOT PERMITTED		B
WITHOUT PRIOR WRITTEN APPROVAL. FABRICATED FRAMING	3.	G
FABRICATED (PRE-ENGINEERED) TRUSSES MAY BE USED FOR ROOF AND/OR FLOOR FRAMING. INSTALL PER MANUFACTURER'S		R 1
RECOMMENDATIONS AND SPECIFICATIONS TRUSS MANUFACTURER SHALL		S
DESIGN TRUSSES FOR ALL LOADS PER IBC, INCLUDING UNBALANCED SNOW LOADS, SNOW DRIFTING, SNOW BUILD UP IN VALLEYS AND ON EAVES, ETC. TRUSS MANUFACTURER SHALL RECOMMEND AND PROVIDE ALL REQUIRED TRUSS BRACING BLOCKING TRUSS TO TRUSS AND TRUSS TO BEAM		C G
TRUSS BRACING, BLOCKING, TRUSS TO TRUSS AND TRUSS TO BEAM CONNECTIONS, ETC. SEE GENERAL TRUSS NOTES. SHOP DRAWINGS FOR ALL FABRICATED FRAMING SHALL BE SUBMITTED FOR	1.	A
REVIEW AND APPROVAL PRIOR TO FABRICATION AND INSTALLATION.		N C

	_ REVIEW AND APPROV	ial prior to fabri	CATION AND
	WOOD TRUS	S NOTES	`.).
	DESIGN LOADS FOR WOOD	ROOF TRUSSES:	
L	TOP CHORD LIVE LOAD TOP CHORD DEAD LOAD BOT CHORD LIVE LOAD BOT CHORD DEAD LOAD	=30PSF =10PSF =0PSF =5PSF <u>ST(</u> 2	<u>) DNE VENEE</u> ATTACH 1
	TOTAL DESIGN LOAD ADDITIONAL WIND LOADING	=45PSF =72PSF_OVER PRESSURE_LOAD	applied (Furring) Bottom F
	DEFLECTION CRITERIA FOR ROOF TRUSSES:	WOOD	THE MESH BENT SO
- -,	LIVE LOAD DEFLECTION TOTAL LOAD DEFLECTION	=L/360 =L/240	THICKNES

ORDINANCES. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE DESIGN AND FABRICATION OF THE PRE-ENGINEERED TRUSSES, PER THE DESIGN CRITERIA ABOVE. DESIGN MUST TAKE INTO ACCOUNT UNBALANCED SNOW LOADS, SNOW DRIFTING, INCREASED SNOW LOADS ON EAVES AND IN VALLEYS, IMPACT LOADS FROM FALLING SNOW AND ICE, ETC. COMPASS ENGINEERING IS NOT RESPONSIBLE FOR THE DESIGN, INSTALLATION, ETC. OF THE TRUSSES. SHOP DRAWINGS FOR ALL WOOD TRUSSES SHALL BE UBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW AND APPROVAL PRIOR TO ABRICATION AND INSTALLATION. TRUSS CONNECTORS SHALL BE SPECIFIED BY THE TRUSS HE TRUSSES SHALL BE DESIGNED TO CARRY ANY ADDITIONAL LOADS DUE TO IECHANICAL UNITS, OVERHEAD DOORS, ROOF OVERBUILDS, ETC. SEE STRUCTURAL PLANS FOR ADDITIONAL REQUIREMENTS. ALL MEMBERS SHALL BE DESIGNED FOR COMBINED STRESSES, BASED ON THE WORST LOADING CONDITION. BOTTOM CHORDS OF TRUSSES, ACTING AS CEILING MEMBERS, MUST BE ABLE TO SUPPORT A 10 PSF LIVE LOAD PER CODE REQUIREMENTS. EACH CHORD SECTION SHALL INVOLVE TWO PANEL POINTS BEFORE BEING SPLICED. PROVIDE 1/8" CAMBER FOR EACH 6 FEET OF TRUSS UNLESS OTHERWISE NDICATE TRUSSES WHICH EXCEED 12'-0" IN HEIGHT MAY REQUIRE A CAP TRUSS IN ORDER TO TRANSPORT. VERIFY WITH TRUSS MANUFACTURER. BEAR ON PANEL POINTS OF SUPPORTING TRUSS. SHEATH OR BLOCK ALL CHORDS TYP. B BEARING POINTS OF GIRDER TRUSSES ARE TO BE SUPPORTED BY A BUILT-UP $_{-}$ column equal to the number of plys in girder truss plus two (2) MINIMUM OF (3) STUDS (U.N.O.).

FABRICATION AND INSTALLATION FABRICATION OF TRUSSES SHALL BE AS APPROVED BY ICC. THIS SPECIFICATION SHALL GOVERN WHEN IT EXCEEDS ICC REQUIREMENTS. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO FABRICATION. FABRICATE TRUSSES FROM SHOP DRAWINGS REVIEWED AND APPROVED BY THE ARCHITECT/ENGINEER. ANY LAYOUT DEVIATION FROM STRUCTURAL DRAWINGS MUST BE APPROVED BY THE ENGINEER.

FABRICATE TRUSSES IN JIGS WITH MEMBERS ACCURATELY CUT TO PROVIDE GOOD BEARING AT JOINTS. JOINTS SHALL BE ACCEPTABLE IF THE AVERAGE OPENING BETWEEN ENDS OF MEMBERS IMMEDIATELY AFTER FABRICATION IS LESS THAN 6", EXCEPT THAT TRUSS COMPRESSION CHORD JOINTS AT SPLICES AND GES SHALL HAVE FULL CONTACT BETWEEN MEMBERS. ISS FABRICATORS USING METAL PLATES SHALL HAVE PLANT INSPECTED FOUR ES PER YEAR BY AN INDEPENDENT TESTING LABORATORY IN ACCORDANCE WITH REGULATIONS AND COPIES OF INSPECTIONS MADE AVAILABLE TO OWNER UPON TE CONTRACTOR SHALL BE RESPONSIBLE FOR THE INSTALLATION OF THE

RUSSES PER THE TRUSS MANUFACTURER'S RECOMMENDATIONS AND PECIFICATIONS. NO WEB OR CHORD MEMBERS SHALL BE MODIFIED IN THE TRUSS BRACING AND BLOCKING THE TRUSS MANUFACTURER SHALL SPECIFY PROPER BRACING OF COMPRESSION

CHORD MEMBERS 6'-0" LONG (OR LONGER), AS WELL AS BRACING REQUIRED FOR TRUSS ERECTION, AND ANY OTHER BRACING. THE TRUSS MANUFACTURER SHALL SPECIFY ALL REQUIRED TRUSS BLOCKING. TRUSS BLOCKING SHALL BE DESIGNED FOR LATERAL LOADINGS.

BOTTOM CHORD OF TRUSSES TO BE SHEATHED W/ 5/8" GYPSUM WALL BOARD OR BRACED WITH CONT. 2X4 @ 6'-0" TRUSS MANUFACTURER TO VERIFY. TOP CHORDS OF TRUSSES SHALL BE SHEATHED WITH ROOF SHEATHING, ELSE BRACE WITH CONT. 2X4 @ 6'-0" O.C.

METAL GUSSET PLATES GUSSET PLATES SHALL BE SPECIFIED FOR GREATER OF EITHER THE MEMBER FORCES SHOWN ON DRAWINGS OR THE MEMBER FORCES DERIVED FROM STRUCTURAL ANALYSIS. PLUS OR MINUS 6%. NO PANEL POINT SHALL HAVE MORE THAN ONE PLATE PER TRUSS SIDE.

PLATES SHALL HAVE MINIMUM BITE OF 2 1/2" ON MEMBERS. MEASURE BITE ALONG CENTER LINE OF WEBS AND PERPENDICULAR TO CHORD AXES. ORIENT PLATE AXIS PARALLEL WITH TRUSS CHORD AXIS EXCEPT WHERE CHORDS CHANGE PITCH AT JOINT.

PLATE SIZES: MINIMUM WIDTH OF PLATES SHALL BE 3". FOR TRUSSES OTHER THAN SCISSOR TRUSSES, DESIGN PLATES, FOR 135% OF MEMBER FORCES. FOR SCISSOR TRUSSES, DESIGN PLATES, FOR 160 % OF MEMBER FORCES. NO NCREASE IN PLATE VALUES WILL BE ALLOWED FOR DURATION OF LOADING OR THFR FACTORS.

RESS PLATES INTO MEMBERS TO OBTAIN FULL PENETRATION WITHOUT CRUSHING UT SURFACE OF WOOD. PLATE EMBEDMENT IS ACCEPTABLE IF OPENING BETWEEN PLATE AND WOOD SURFACE IS LESS THAN 1/32". A COMBINATION OF LUMBER DEFECTS AND PLATE MISPLACEMENT SHALL NOT REDUCE PLATE AREA OR NUMBER OF EFFECTIVE TEETH, PRONGS, OR NAILS BY MORE THAN 10%.

DO NOT APPLY METAL GUSSET PLATES AFTER SHOP FABRICATION. ENEER:

OTHER METHODS OF ATTACHMENT MAY BE USED WITH WRITTEN PERMISSION FROM THE ARCHITECT AND STRUCTURAL ENGINEER. PROVIDE STEEL ANGLE LINTELS AT ALL OPENINGS. SEE THE STEEL ANGLE LINTEL SCHEDULE FOR SIZE AND REQUIREMENTS. <u>BRICK VENEER</u>

ATTACH TO STEEL AND WOOD STUD WALLS WITH DUR-O-WAL DA 213 SEISMIC /ENEER ANCHORS OR HOHMANN & BARNARD DW-10 OR DW-10HS SEISMIC VENEER ANCHORS SPACED AT 16" O.C. EACH WAY. ATTACH VENEER ANCHORS TO STUDS WITH #10 CORROSION RESISTANT SELF-DRILLING SCREWS. ATTACH THE VENEER TO THE ANCHORS WITH DUR-O-WAL SEISMIC STEEL PINTELS OR HOHMANN & BARNARD 6" DIAMETER BYNA-TIE WITH SEISMICLIPS. ANCHOR TIES MUST ENGAGE A

CONTINUOUS, HORIZONTAL GALVANIZED #9 GAUGE WIRE PLACED IN THE CENTER OF THE VENEER AT 16" O.C. (MAX.). ATTACH TO CONCRETE WALLS WITH 22 GAUGE GALVANIZED, VERTICAL DOVETAIL SLOTS AND DUR-O-WAL 16 GAGE SEISMIC DOVETAIL ANCHOR TIES OR HOHMANN & ARNARD 3/16" DIAMETER BYNA—TIE WITH SEISMICLIPS SPACED AT 16" O.C. (MAX. IN EACH DIRECTION. ANCHOR TIES MUST ENGAGE A HORIZONTAL GALVANIZED #9 GAUGE WIRE PLACED IN THE CENTER OF THE VENEER AT 16" O.C. (MAX). ATTACH TO REINFORCED MASONRY WALLS WITH TRI-ROD LADDER TYPE

eįNFORCĘMENT WITH THREE #9 GAUGE GALVANIZED CORRUGATED WIRES SPACED AT " O.C. (MAX.) VERTICALLY. "OPTION: ATTACH VENEER WITH DUR-O-WAL DA 3605 EISMIC LÀDDER-EYE SPACED AT 16" O.C. (MAX.) IN EACH DIRECTION. ANCHOR TIES MUST ENGAGE A HORIZONTAL GALVANIZED #9 GAUGE WIRE PLACED IN THE CENTER OF THE VENEER AT 16" O.C. (MAX). ANCHORS MUST EXTEND INTO THE GALVANIZED LADDER TYPE JOINT REINFORCEMENT IN THE MASONRY WALL.

STONE VENEER NOTES (PART 1) ATTACH TO CONCRETE OR MASONRY BACKING, WITH 12 GAUGE MIN. GALVANIZED WIRE, FORMED BEYOND THE BASE OF THE BACKING. THE LEGS OF THE LOOPS SHALL BE 6" MIN IN LENGTH BENT AT RIGHT ANGLES AND LAID IN THE MORTAR JOINT, AND SPACED SO THAT THE EYES OR LOOPS ARE 12" MAX. ON CENTER IN BOTH DIRECTIONS. THERE SHALL BE A 12 GAUGE MIN. GALVANIZED WIRE TIE THREADED THROUGH THE EXPOSED LOOPS FOR EVERY 2 SQUARE FEET OF STONE VENEER. THIS TIE SHALL BE A LOOP HAVING LEGS 15," MIN. LENGTH BENT SO THAT IT WILL LIE IN THE MORTAR JOINT. THE LAST 2" OF EACH WIRE LEG SHALL HAVE A 90° BEND. 1" MIN. THICKNESS OF CEMENT GROUT SHALL BE PLACED BETWEEN THE BACKING AND THE STONE VENEER.

<u>R NOTES (PART 2)</u>
BETWEEN THE BACKING AND THE STONE VENEEN.
IO STUD BACKING WITH A 2"X2"X16 GALVANIZED WIRE MESH WITH TWO LAYERS OF WATERPROOF PAPER BACKING DIRECTLY TO STUDS AT 16" O.C. MAX., THE MESH SHALL BE ATTACHED WITH 2" LONG GALVANIZED STEEL WIRE NAILS AT 4" O.C. WITH 1 1/8" MIN PENETRATION INTO STUDS AND 8d COMMON NAILS AT 8" O.C. INTO TOP AND PLATES OR WITH EQUIVALENT WRE TIES. THERE SHALL BE A 12 GAUGE MIN. GALVANIZED WRE LOOPED THROUGH SH FOR EVERY 2 SQUARE FEET OF STONE VENEER. THIS TIE SHALL BE A LOOP HAVING LEGS 15" MIN. LENGTH D THAT IT WILL LIE IN THE MORTAR JOINT. THE LAST 2" OF EACH WIRE LEG SHALL HAVE A 90° BEND. 1" MIN. SS OF CEMENT GROUT SHALL BE PLACED BETWEEN THE BACKING AND THE STONE VENEER.

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MASONRY LINTEL SCHEDULE								
LINTEL LINTEL LINTEL REINFORCING MARK WIDTH DEPTH HORIZONTAL STIRRUPS								
ML-1	8"	2'-0"	(2) # 5 TOP & BOTTO M	#3 @ 24" O.C.	GR			
ML-2 8" 1'-4" (2) #4 TOP & BOTTOM					GR			
TIES W/ HOOKED ENDS, SEE SCHEDULE								

BOTTOM REINFORCING, SEE SCHEDULE
MASONRY LINTEL NOTES:

LINTEL SHALL MATCH WALL WIDTH AND MATERIAL TYPE. U.N.O. SOLID GROUT MASONRY LINTELS MONOLITHICALLY WITH THE WALL OR COLUMN AT EACH END. SHORE AS REQUIRED. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS THAT ARE LOCATED DIRECTLY BELOW FLOOR / ROOF BEAMS OR GIRDERS. 5. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS MINIMUM BEYOND THE EDGE OF ALL OPENINGS PROVIDE 90° STANDARD HOOK. IF HORIZONTAL REINFORCING CANNOT EXTEND 48 BAR DIAMETERS BEYOND EDGE OF OPENING. 6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY. 7. CONTINUE HORIZONTAL WALL REINFORCING THRU MASONRY LINTELS. USE LARGER REINFORCING WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE. 8. DOWEL VERTICAL WALL REINFORCING INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

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$\mathbb{A}^{\mathcal{F}}_{\mathcal{F}}$						MASONRY	Ŵ,
>	Γ	WALL	THICKNESS		SOLID		REIN
$\langle \rangle$		MARK	THICKNESS	MATERIALS	GROUT	VERTICAL	

>						
>	WALL	THICKNESS	MATERIALS	SOLID		REIN
>	MARK	THURINESS	MATERIALS	GROUT	VERTICAL	
>						
>	MW-1	8"	CMU	NO	(1) #5 @ 24" O.C. E.F.	
>	MW-2	8"	CMU	NO	(1) #5 @ 24" O.C.	
$\langle \cdot \rangle$						

1 COORDINATE WITH ARCHITECTURAL DRAWINGS, MASONRY WALL FINISHES, TYPES OF MATERIAL, COURSING, ETC... 2. DO NOT SOLID GROUT WALLS UNLESS NOTED OTHERWISE.

3. SOLID GROUT ALL MASONRY BELOW GRADE.

4. CENTER VERTICAL REINFORCING IN THE WALL UNLESS NOTED OTHERWISE.

5. PLACE HORIZONTAL WALL REINFORCING BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS. 6. CONTINUE HORIZONTAL WALL REINFORCING THRU MASONRY LINTELS. USE LARGER REINFORCING WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE.

7. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

8. MASONRY WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS.

9. f'm= 2500 PSI

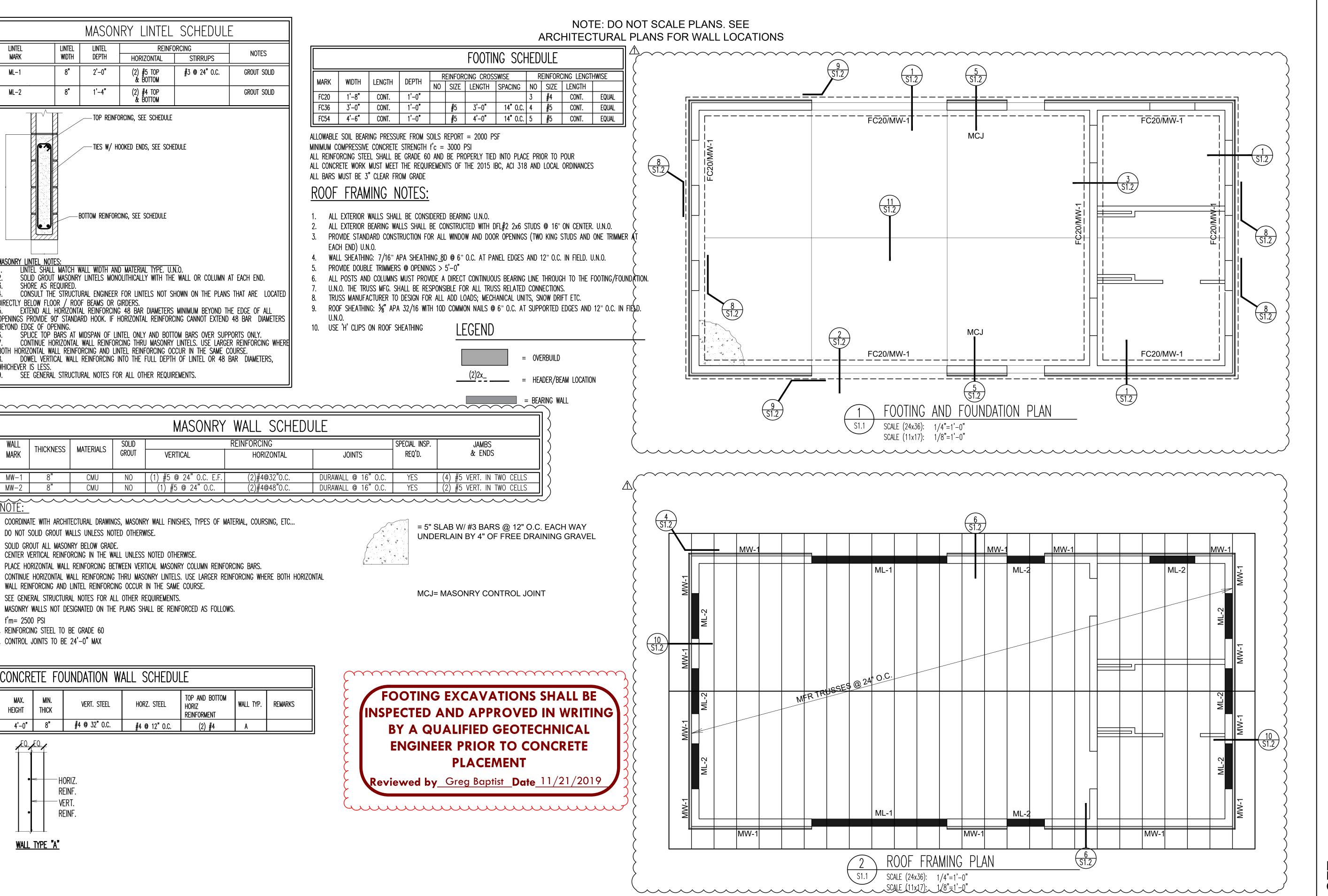
10. REINFORCING STEEL TO BE GRADE 60

11. CONTROL JOINTS TO BE 24'-0" MAX

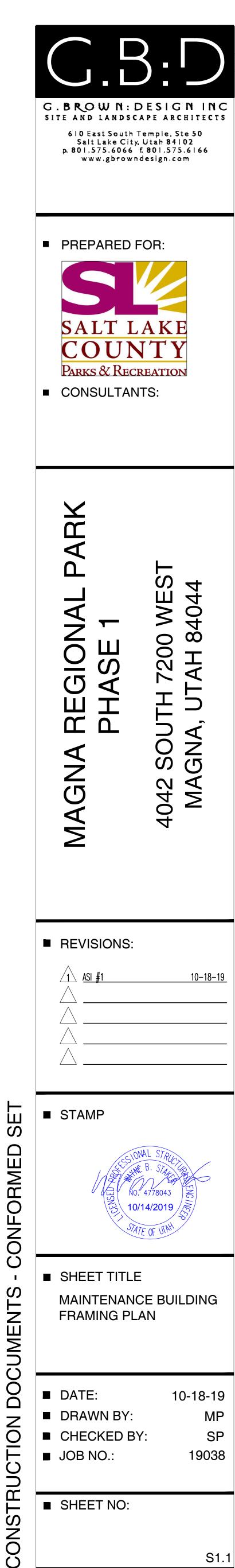
I REINFORMENT	Max. Height		VERT. STEEL	Horz. Steel	Top and bottom Horiz Reinforment	WA
4'-0" 8" #4 @ 32" 0.C. #4 @ 12" 0.C. (2) #4	4'-0"	² -0" 8"	#4 @ 32" 0.C.	#4 @ 12" O.C.	(2) #4	

ŁQ. ŁQ. / - HORIZ REINF. - VERT. REINF.

<u>wall type "a"</u>

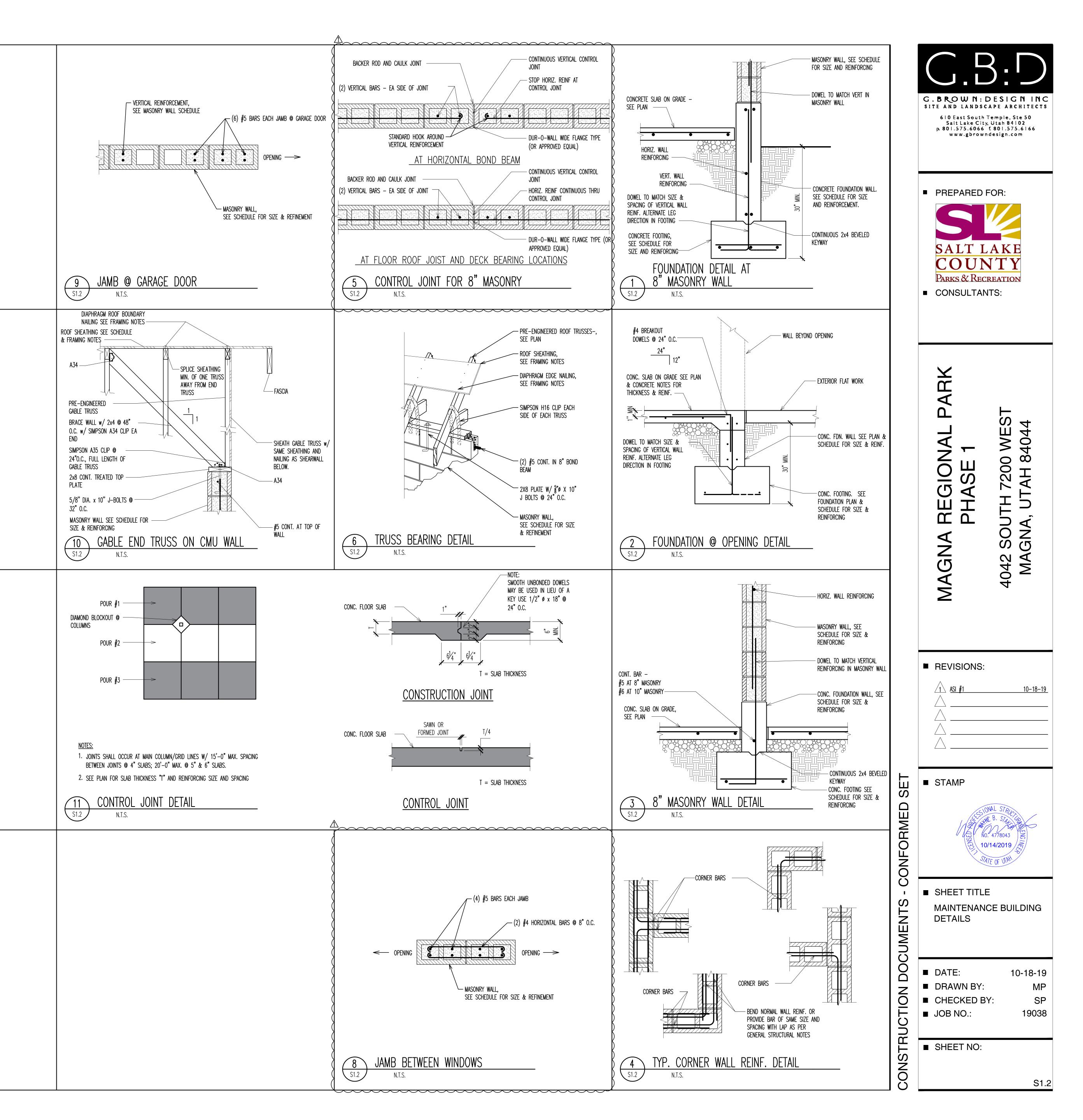


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XIBUILDING XISTRUCTURAL XIMECHANICAL PLUMBING XIELECTRICAL ENERGY XACCESSIBILITY FIRE PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS. BY: DATE: 11/13/19	REVIEWED FOR CODE	
XIBUILDING XISTRUCTURAL XIMECHANICAL PLUMBING XIELECTRICAL ENERGY XACCESSIBILITY FIRE PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS. BY: DATE: 11/13/19	COMPLIANCE FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW	
X ELECTRICAL X ENERGY X ACCESSIBILITY FIRE PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS. BY: DATE: 11/13/19	X BUILDING X STRUCTURAL	
PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS. BY:		
DOES NOT AUTHORIZE CONSTRUCTION TO PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS. BY: DATE: 11/13/19		
BY: <u>ALLAN</u> DATE: 11/13/19	PLAN REVIEW ACCEPTANCE OF DOCUMENTS DOES NOT AUTHORIZE CONSTRUCTION TO	
	PROCEED IN VIOLATION OF ANY FEDERAL, STATE, OR LOCAL REGULATIONS.	
	WEST COAST CODE CONSULTANTS, INC.	
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019 Jobs/19038 Maintenance Buildings/Cad/Structura/01903





MASONRY LINTEL SCHEDULE							
LINTEL MARK	lintel Width	lintel Depth	REINFC HORIZONTAL)rcing Stirrups	NOTES		
ML-1	8"	2'-0"	(2) # 5 TOP & BOTTO M	#3 @ 24" O.C.	grout solid		
ML-2	8"	1'-4"	(2) #4 TOP & BOTTOM		grout solid		
3. SHORE AS RI 4. CONSULT THE DIRECTLY BELOW FLOO 5. EXTEND ALL OPENINGS PROVIDE 90 BEYOND EDGE OF OP 6. SPLICE TOP 7. CONTINUE HO BOTH HORIZONTAL WA	Es: Match V Masonr) Equired. Structu Dr / Roc Horizonti O' Standa Ening. Bars at I Prizontal LL Reinfo	TIES W/ HO BOTTOM REINFORC VALL WIDTH ANI VALL WIDTH ANI VIDSPAN OF LIN WIDSPAN OF LIN WALL REINFOR WIDSPAN OF LIN WALL REINFOR WALL REINFOR	3 48 BAR DIAMETERS IORIZONTAL REINFORC NTEL ONLY AND BOTT CING THRU MASONRY	N.O. Wall or column / Hown on the plans Minimum beyond th Ding cannot extend OM bars over supp Lintels. Use large CCUR in the same c	5 THAT ARE LOCAT 1E EDGE OF ALL 48 BAR DIAMETEI PORTS ONLY. R REINFORCING WH OURSE.		

WHICHEVER IS LESS. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.

		MASONRY WALL						
	WALL MARK	THICKNESS	MATERIALS	solid Grout	VERTICAL	REINFORC HO		
$\left\{ \right\}$	MW-1 MW-2	8" 8"	CMU CMU	NO NO	(1) #5 @ 24" O.C. E.F. (1) #5 @ 24" O.C.	(2)#4 (2)#4		
				$\overline{}$				

NOTE:

1. COORDINATE WITH ARCHITECTURAL DRAWINGS, MASONRY WALL FINISHES, TYPES OF MATERIAL, COURSING, ETC...

2. DO NOT SOLID GROUT WALLS UNLESS NOTED OTHERWISE.

3. SOLID GROUT ALL MASONRY BELOW GRADE.

4. CENTER VERTICAL REINFORCING IN THE WALL UNLESS NOTED OTHERWISE.

5. PLACE HORIZONTAL WALL REINFORCING BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS. 6. CONTINUE HORIZONTAL WALL REINFORCING THRU MASONRY LINTELS. USE LARGER REINFORCING WHERE BOTH HORIZONTAL

WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE.

7. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS. 8. MASONRY WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS.

9. f'm= 2500 PSI

10. REINFORCING STEEL TO BE GRADE 60

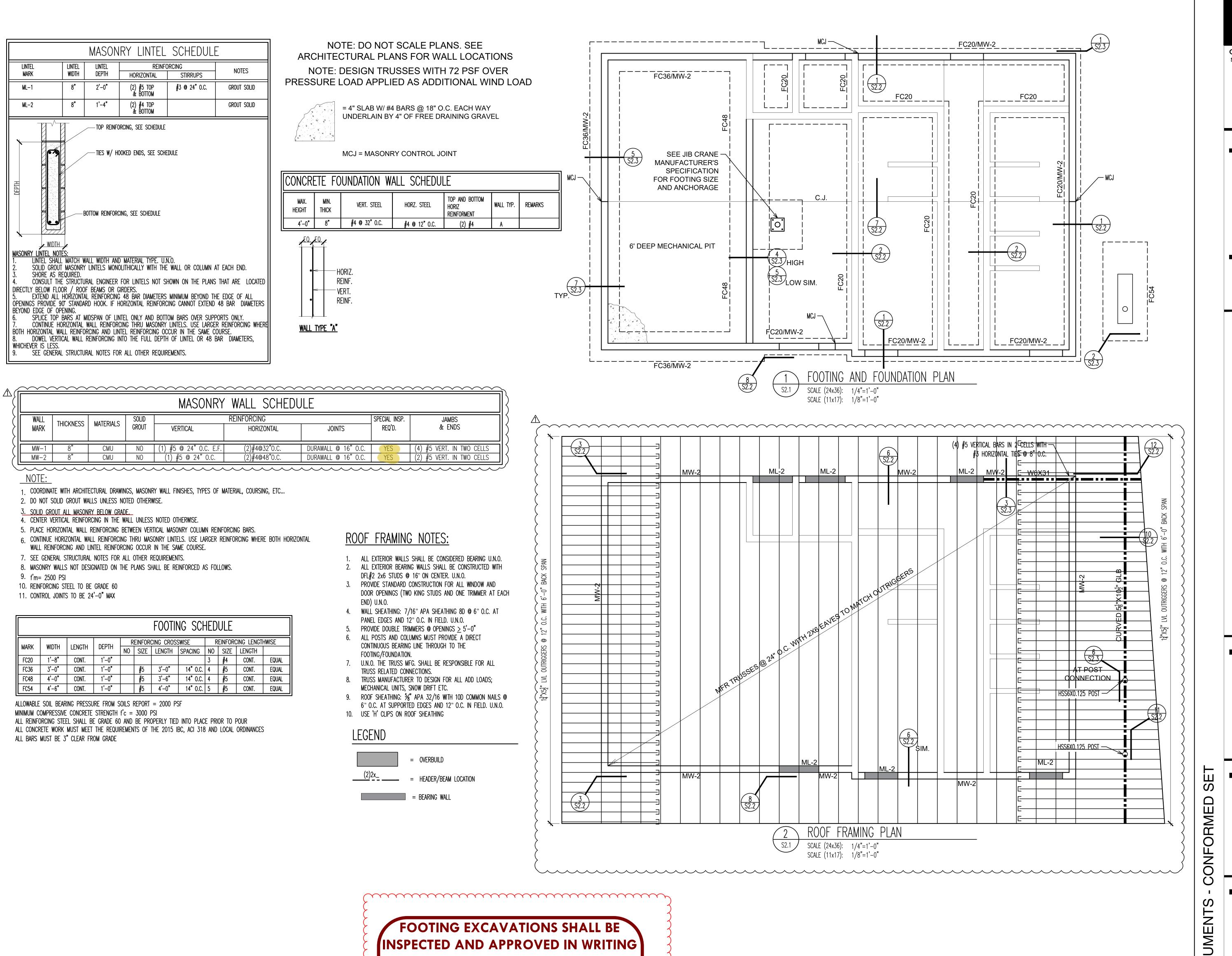
11. CONTROL JOINTS TO BE 24'-0" MAX

						FOOTIN	NG SCH	HED	ULE	
	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LEN		
MARK	WIDTH			NO	SIZE	LENGTH	SPACING	NO	SIZE	LENGTH
FC20	1'-8"	CONT.	1'-0"					3	#4	CONT.
FC36	3'-0"	CONT.	1'-0"		# 5	3'-0"	14" 0.C.	4	# 5	CONT.
FC48	4'-0"	CONT.	1'-0"		# 5	3'-6"	14" O.C.	4	# 5	CONT.
FC54	4'-6"	CONT.	1'-0"		# 5	4'-0"	14" O.C.	5	# 5	CONT.

ALLOWABLE SOIL BEARING PRESSURE FROM SOILS REPORT = 2000 PSF

MINIMUM COMPRESSIVE CONCRETE STRENGTH f'c = 3000 PSI

ALL REINFORCING STEEL SHALL BE GRADE 60 AND BE PROPERLY TIED INTO PLACE PRIOR TO POUR ALL CONCRETE WORK MUST MEET THE REQUIREMENTS OF THE 2015 IBC, ACI 318 AND LOCAL ORDINANCES ALL BARS MUST BE 3" CLEAR FROM GRADE

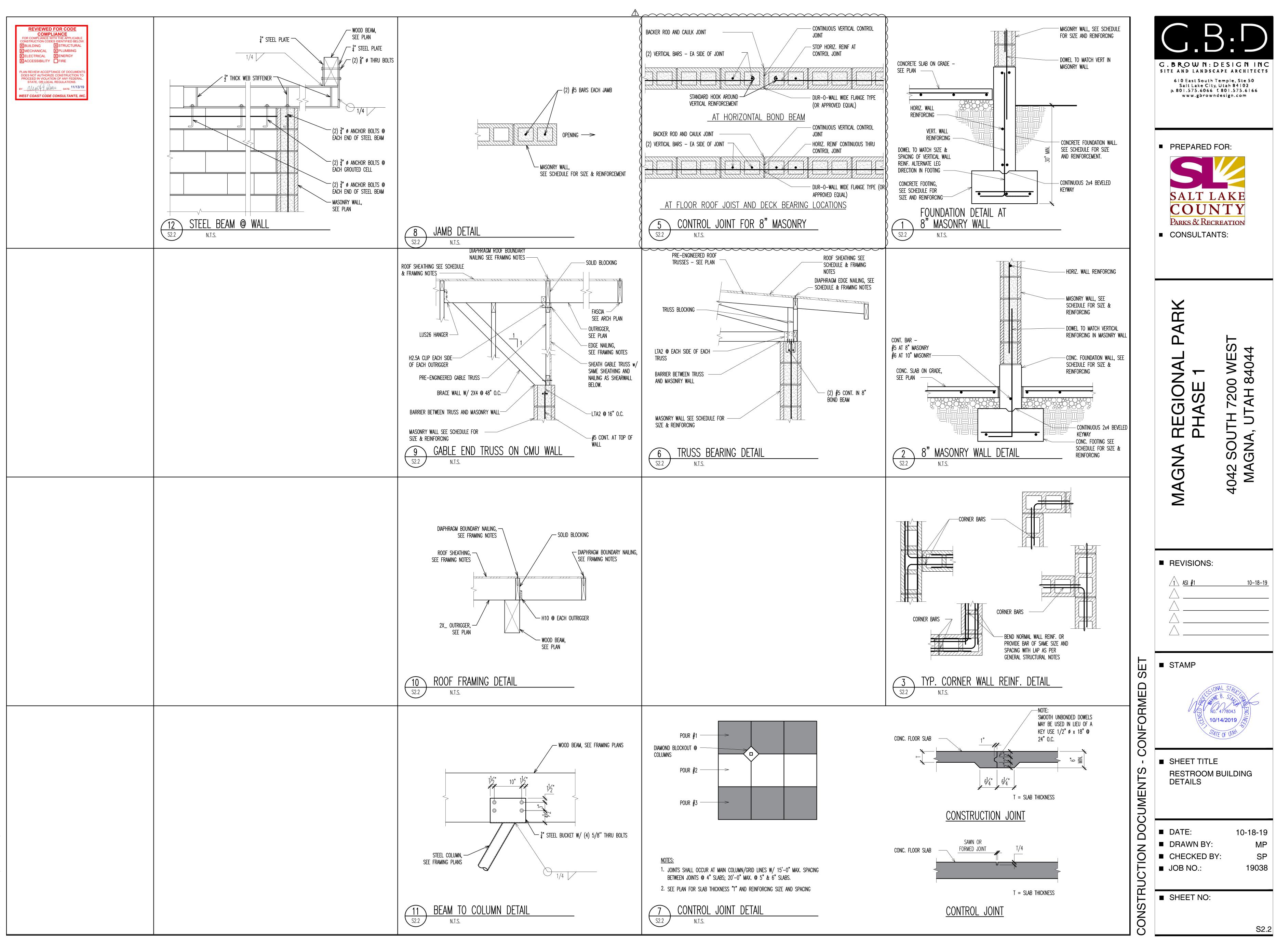


INSPECTED AND APPROVED IN WRITING BY A QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO CONCRETE PLACEMENT

Reviewed by Greg Baptist Date 11/21/2019

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COMPLIANCE FOR COMPLIANCE WITH THE APPLICABLE CONSTRUCTION CODES IDENTIFIED BELOW. Image: Im	

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