

# NEW CANOPY FOR AMERICAN FORK DESERET INDUSTRIES

435 SOUTH 500 EAST - AMERICAN FORK, UT

## OWNER:

THE CHURCH OF JESUS CHRIST  
OF LATTER-DAY SAINTS  
TOM HOWELL, PROJECT MANAGER

AMERICAN FORK PROJECT MANAGEMENT OFFICE  
P.O. BOX 268  
AMERICAN FORK, UT 84003

PH. # (801) 763-4520 FAX# (801) 763-4548

## ARCHITECT:

RVA ARCHITECTS, INC.  
TRAVIS DANCE, PROJECT ARCHITECT

32 W. CENTER ST., SUITE 203  
PROVO, UTAH 84601

PH. # (801) 374-2100

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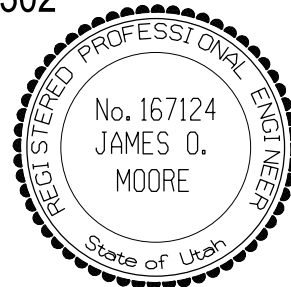
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## STRUCTURAL ENGINEER

### MOUNTAIN VIEW ENGINEERING

JAMES O. MOORE  
345 N. MAIN, SUITE A  
BRIGHAM CITY, UTAH 84302

PH. (435) 734-9700



## ELECTRICAL ENGINEER

### ROYAL ENGINEERING

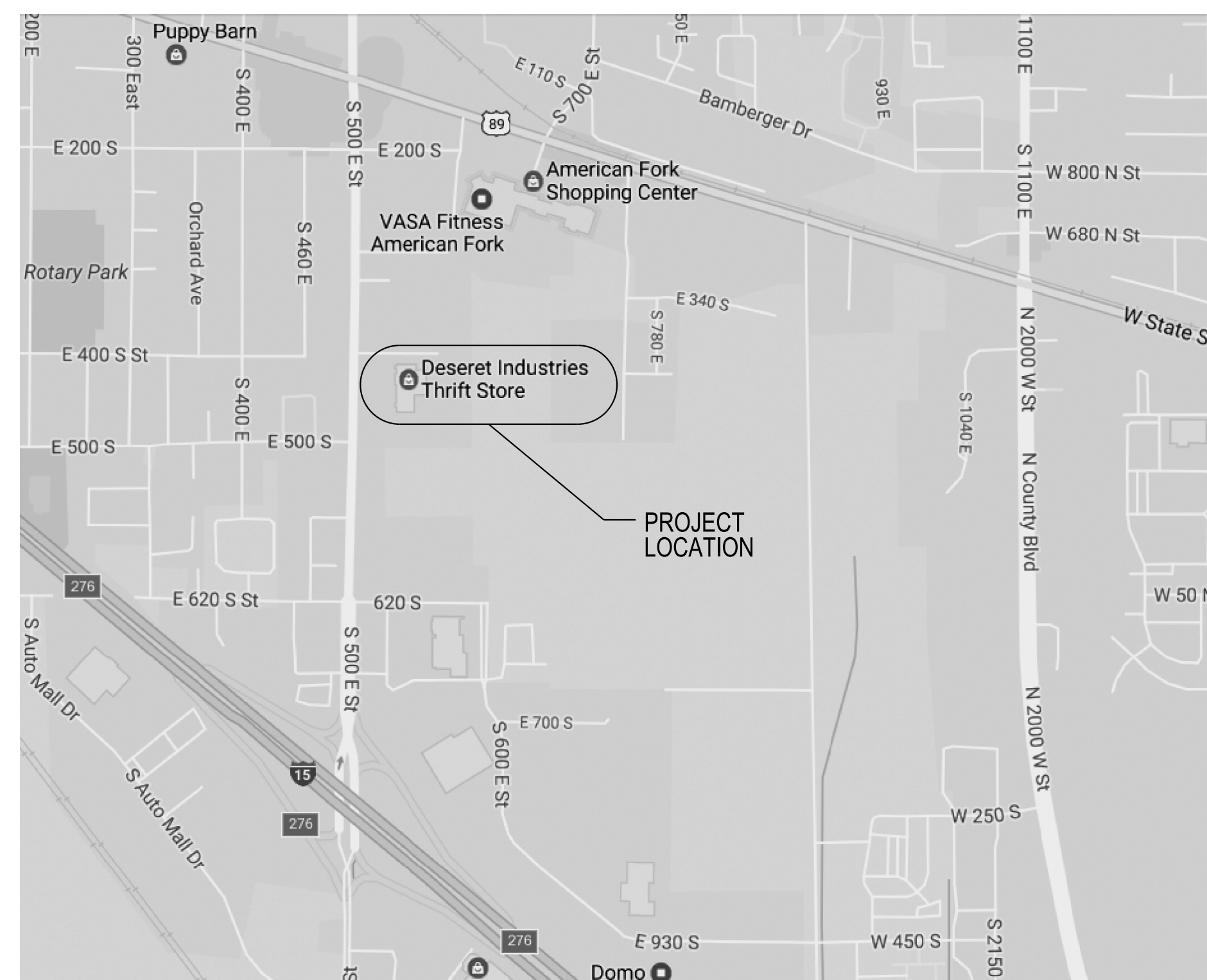
STEVEN R. GRIFFITHS  
1837 S. EAST BAY BLVD.  
PROVO, UTAH 84606

PH. (801) 375-2228

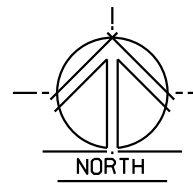


## CODE INFORMATION

2015 IBC, 2014 NEC  
OCCUPANCY: U  
CONSTRUCTION TYPE: II-B  
BUILDING AREA: 3,600 S.F.



AMERICAN FORK VICINITY MAP



NEW CANOPY FOR  
**AMERICAN FORK  
 DESERET INDUSTRIES**  
 435 SOUTH 500 EAST - AMERICAN FORK, UT

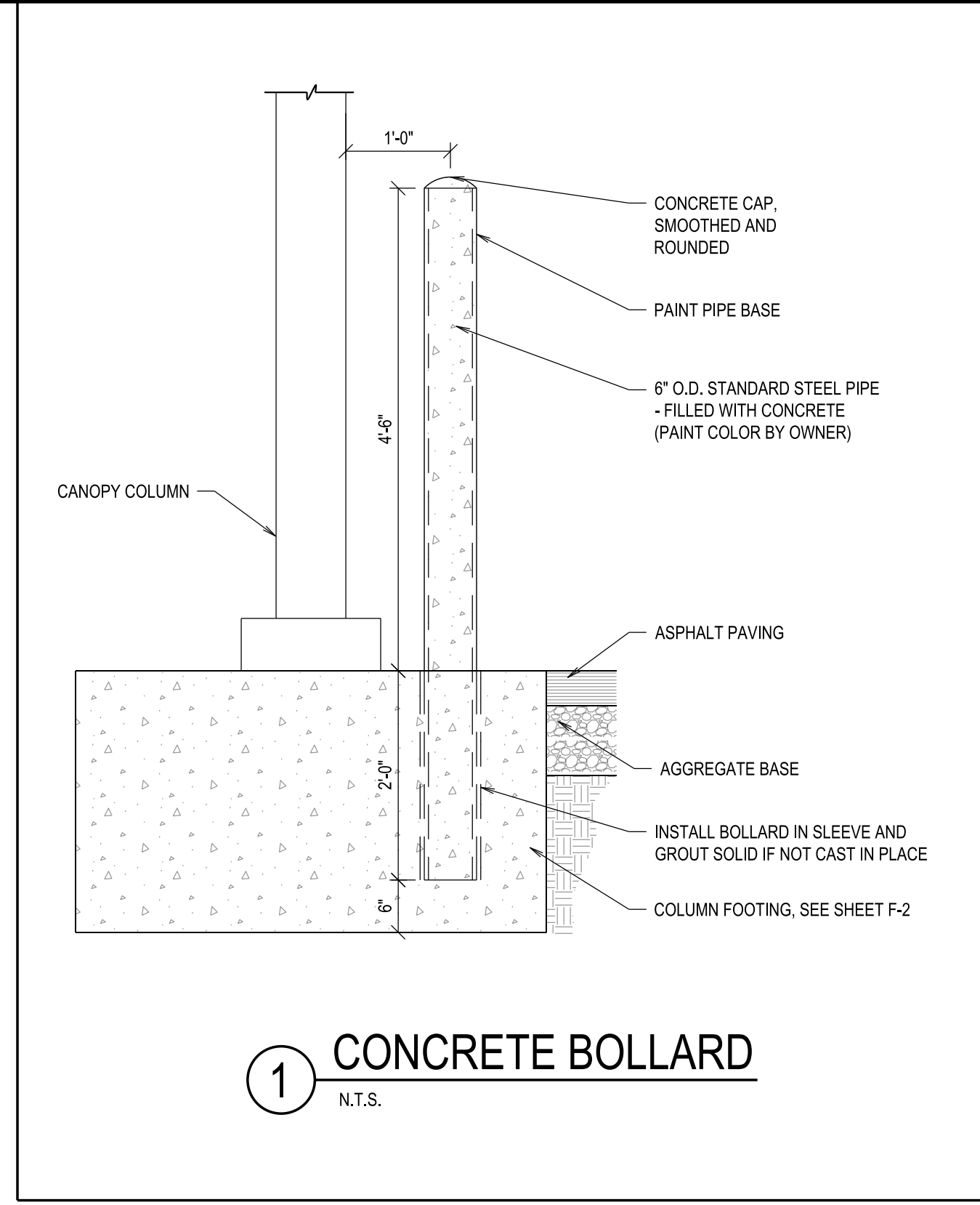
PROJECT FOR  
**THE CHURCH OF  
 JESUS CHRIST  
 OF LATTER-DAY SAINTS**

Mark	Date (MM/DD)	Description

Project Number: 17-20  
 Plan Series:  
 Property Number: 559-7609  
 Date: 10/31/17

SHEET TITLE:  
**COVER SHEET**

SHEET:  
**G-1.1**



- KEYED NOTES:
- 01 OUTLINE OF NEW CANOPY.
  - 02 CANOPY COLUMN.
  - 03 BOLLARD. SEE DETAIL 1/SD-1.1.
  - 04 SAWCUT & REMOVE EXISTING 4" ASPHALT PAVING AS NECESSARY FOR NEW CONDUIT TRENCH, PATCH & REPAIR ASPHALT.
  - 05 SAWCUT & REMOVE EXISTING 6" CONCRETE PAVING AS NECESSARY FOR NEW CONDUIT TRENCH, PATCH & REPAIR CONCRETE.
  - 06 SAWCUT & REMOVE EXISTING 4" ASPHALT PAVING AS NECESSARY FOR FOOTING EXCAVATION, PATCH & REPAIR ASPHALT.
  - 07 CORE DRILL EXISTING CMU EXTERIOR WALL FOR NEW ELECTRICAL CONDUIT PENETRATION. ROUTE CONDUIT DOWN WALL SURFACE TO TRENCH, PAINT EXPOSED CONDUIT TO MATCH EXTERIOR WALL.
  - 08 REMOVE EXISTING PAINTED STRIPING.
  - 09 NEW PAINTED STRIPING.
  - 10 APPLY NEW COAT OF PAINT TO EXISTING STRIPING, LETTERING, & ARROWS IN DONATIONS AREA.
  - 11 EXISTING STORM DRAIN CATCH BASIN TO REMAIN.
  - 12 EXISTING MANHOLE TO REMAIN.
  - 13 EXISTING 12" CONCRETE STORM DRAIN PIPING TO REMAIN. DEPTH IS UNKNOWN.
  - 14 EXISTING 8" PVC STORM DRAIN PIPING TO REMAIN. DEPTH IS UNKNOWN.
  - 15 EXISTING LIGHT POLE TO REMAIN.
  - 16 APPROXIMATE LOCATION OF EXISTING ASPHALT GRADE BREAK.
  - 17 EXISTING ASPHALT DRAINAGE DIRECTION.
  - 18 SAWCUT & REMOVE PORTION OF EXISTING CONCRETE CURB & GUTTER AS NECESSARY FOR FOOTING EXCAVATION, PATCH & REPAIR CURB & GUTTER.
  - 19 EXISTING CURB & GUTTER TO REMAIN.
  - 20 PROPERTY LINE.
  - 21 PAINT NEW ELECTRICAL CONDUIT TO MATCH EXISTING.

- GENERAL NOTES:
1. CANOPY STRUCTURE IS TO BE PROVIDED BY PRECISION CANOPY, 1497 N. 40 S., LINDON, UT 84042, 800-924-2580. COST TO BE PAID BY GENERAL CONTRACTOR - SEE SPECIFICATION SECTION 01 2100.
  2. FOUNDATION ANCHOR BOLTS TO BE PROVIDED BY PRECISION CANOPY, INSTALLED BY GENERAL CONTRACTOR.
  3. FOOTINGS AND FOUNDATION TO BE PROVIDED BY GENERAL CONTRACTOR. SEE SHEETS F-1 AND F-2.
  4. PAINTING OF CANOPY COLUMNS, BEAMS AND FRAMING MEMBERS TO BE PROVIDED BY GENERAL CONTRACTOR.
  5. IF TEMPERATURES ARE TOO LOW TO REPAIR PAVING, TEMPORARILY FILL EXCAVATION WITH COMPACTED MATERIAL TO CREATE A DRIVEABLE SURFACE UNTIL SUCH TIME AS PAVING CAN BE INSTALLED.
  6. DIMENSIONS ARE TO FACE OF CURB.
  7. EXISTING STORM DRAIN LOCATIONS SHOWN ARE APPROXIMATE. FIELD VERIFY. NOTIFY ARCHITECT IF CONFLICTS WITH FOOTINGS ARISE.

**SITE PLAN**  
SCALE: 1" = 20'-0"  
NORTH

**Architects**  
RVA - Architects + Inc.  
ARCHITECTS + PLANNERS

9167790-1300  
Travis W. Dance  
State of Utah

NEW CANOPY FOR  
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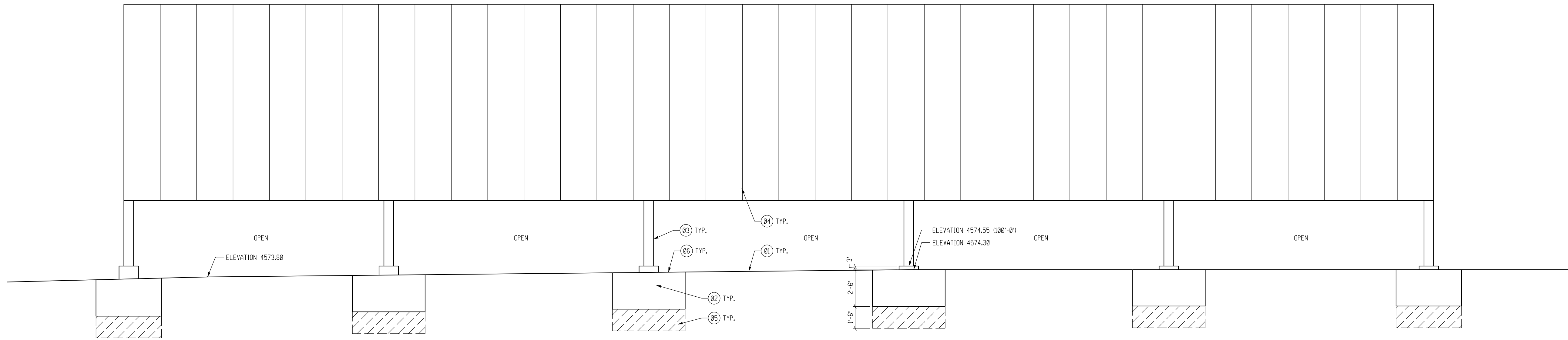
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SHEET TITLE:  
**SITE PLAN**

SHEET:  
**SD-1.1**



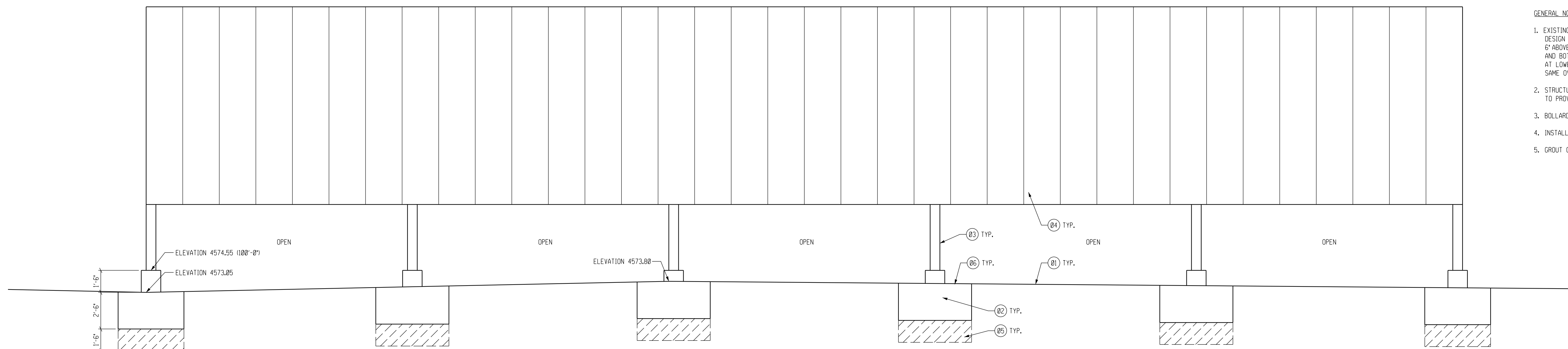
**1 EAST ELEVATION**  
SCALE: 1/4" = 1'-0"

KEYED NOTES:

- (01) EXISTING ASPHALT.
- (02) NEW CONCRETE FOOTING AND FOUNDATION PIER.
- (03) NEW CANOPY COLUMN.
- (04) NEW PRE-FINISHED CANOPY WALL CLADDING.
- (05) NEW STRUCTURAL FILL.
- (06) SLOPE TOP OF FOOTING TO MATCH SLOPE OF ADJACENT ASPHALT.

GENERAL NOTES:

1. EXISTING GRADE ELEVATIONS SHOWN ARE APPROXIMATE. DESIGN INTENT IS THAT TOP OF FOUNDATION PIERS ARE 6" ABOVE TOP OF ASPHALT AT HIGHEST GRADE ELEVATION AND BOTTOM OF FOOTINGS ARE 30" BELOW TOP OF ASPHALT AT LOWEST GRADE ELEVATION. ALL FOUNDATIONS TO BE SAME OVERALL HEIGHT.
2. STRUCTURAL FILL TO BE COMPACTED TO 95%. CONTRACTOR TO PROVIDE PROCTOR TEST AS NECESSARY.
3. BOLLARDS NOT SHOWN FOR CLARITY.
4. INSTALL EXPANSION JOINT BETWEEN ASPHALT AND FOOTING.
5. GROUT COLUMN BASES.



**2 WEST ELEVATION**  
SCALE: 1/4" = 1'-0"



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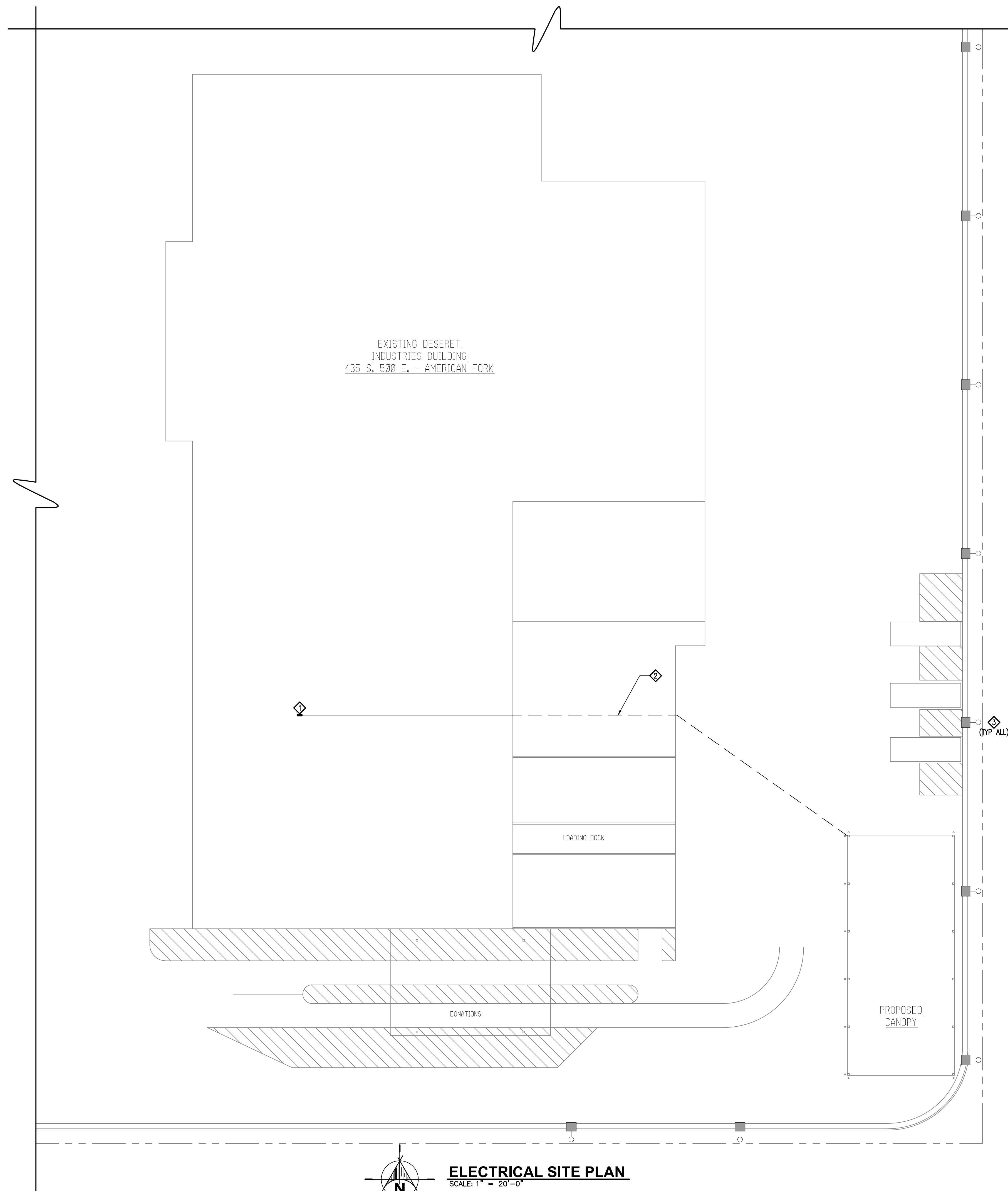
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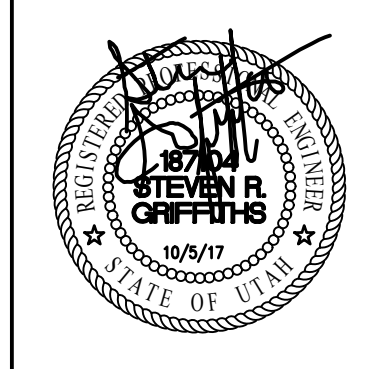
Project Number:  
17-20  
Plan Series:  
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559-7609  
Date:  
10/31/17

SHEET TITLE:  
**ELEVATIONS**





- ELECTRICAL KEYED NOTES:**
- ◇ EXISTING PANEL HA AND LIGHTING CONTROLS LOCATED ON MEZZANINE IN MAIN WAREHOUSE AREA.
  - ◇ NEW LIGHTING CIRCUIT TO CANOPY APPROXIMATELY 250'. USE EXISTING CONDUIT AS AVAILABLE.
  - ◇ EXISTING POLE LIGHT. SHOWN FOR REFERENCE.



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Mark	Date	Description

Project Number: 17-20  
Plan Series:  
Property Number: 559-7609  
Date: 10/2/17

SHEET TITLE:  
**ELECTRICAL  
SITE PLAN**

SHEET:  
**E-1**

**RE**  
**ROYAL ENGINEERING**

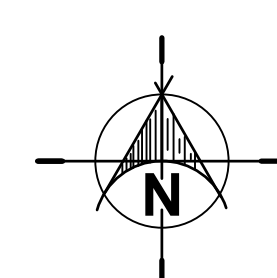
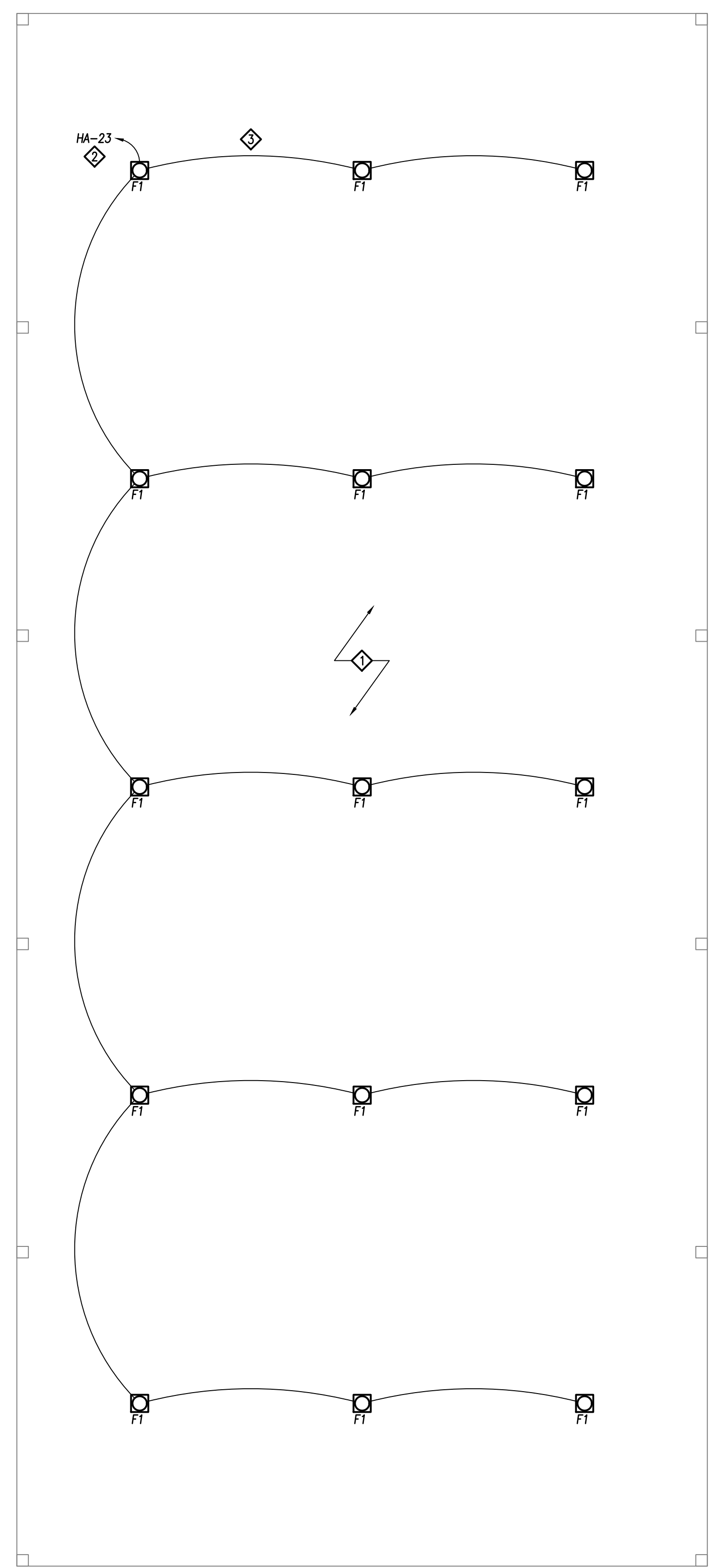
ELECTRICAL MECHANICAL  
1837 S. EAST BAY BLVD. PROVO, UTAH 84606  
PHONE: 801.576.2228 FAX: 801.576.3578

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**ELECTRICAL KEYED NOTES:**

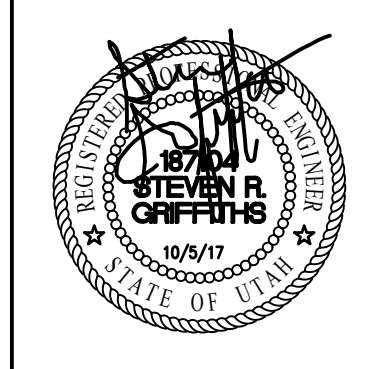
- ◇ LIGHT FIXTURES INCLUDED IN CANOPY KIT. COORDINATE WITH CANOPY INSTALLER PRIOR TO ROUGH-IN.
- ◇ LIGHTS CONTROLLED BY EXISTING TIME CLOCK. SEE DETAIL 1 ON SHEET E-6.
- ◇ ALL WIRING AND CONNECTIONS ARE TO BE MADE BY THE ELECTRICIAN.



**CANOPY LIGHTING PLAN**

SCALE: 3/16" = 1'-0"

**Architects**  
RVA • Architects • Inc.  
ARCHITECTS • PLANNERS



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Project Number:  
17-20

Plan Series:

Property Number:  
559-7609

Date:  
10/2/17

SHEET TITLE:  
**CANOPY  
LIGHTING PLAN**

SHEET:  
**E-2**

**RE**  
**ROYAL ENGINEERING**

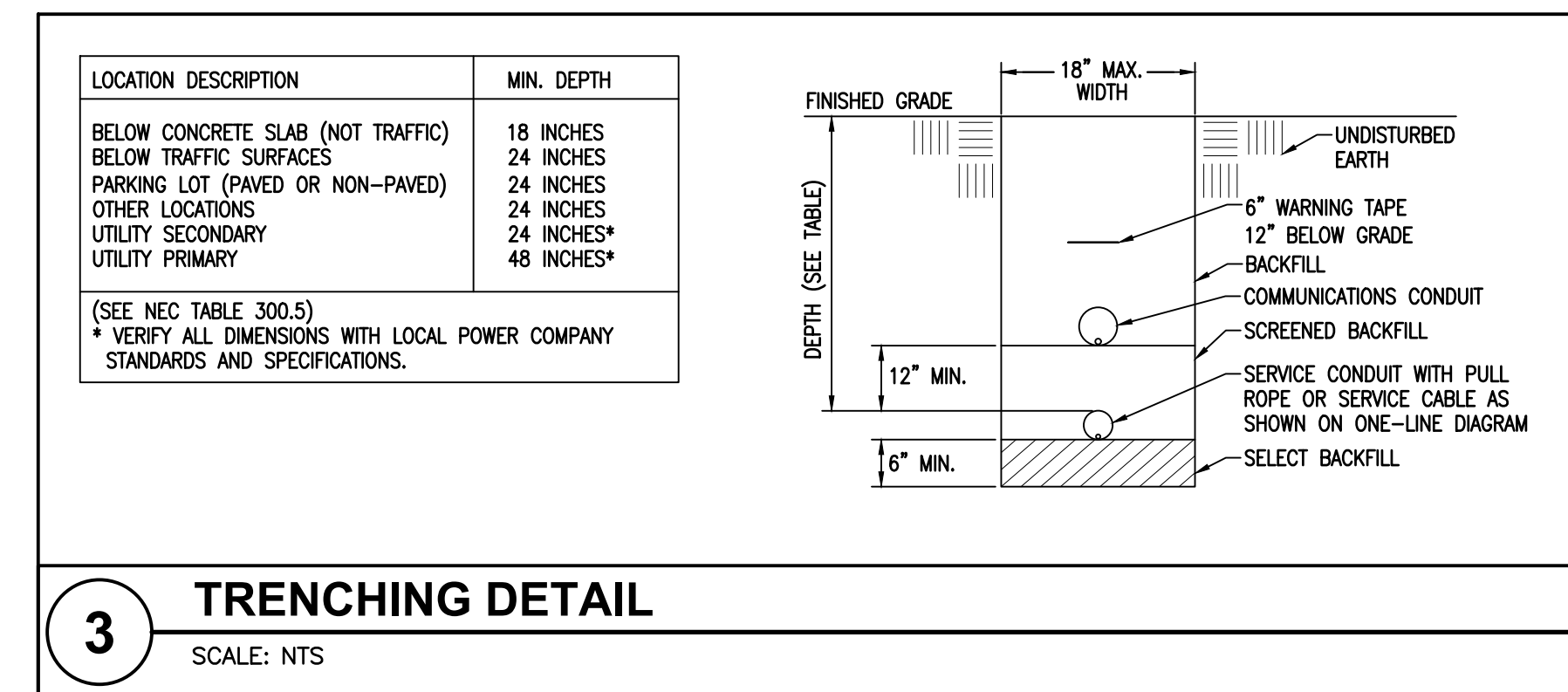
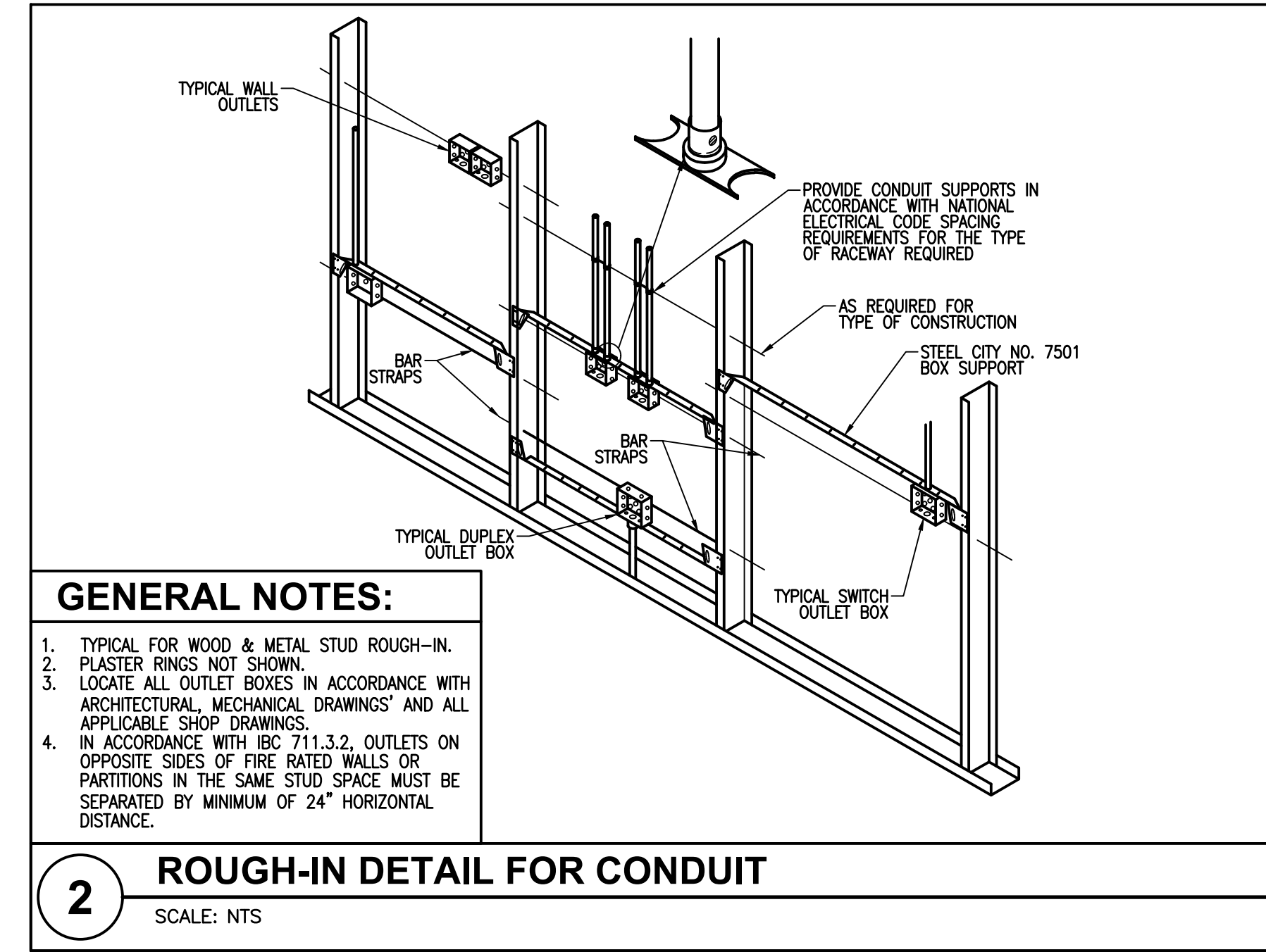
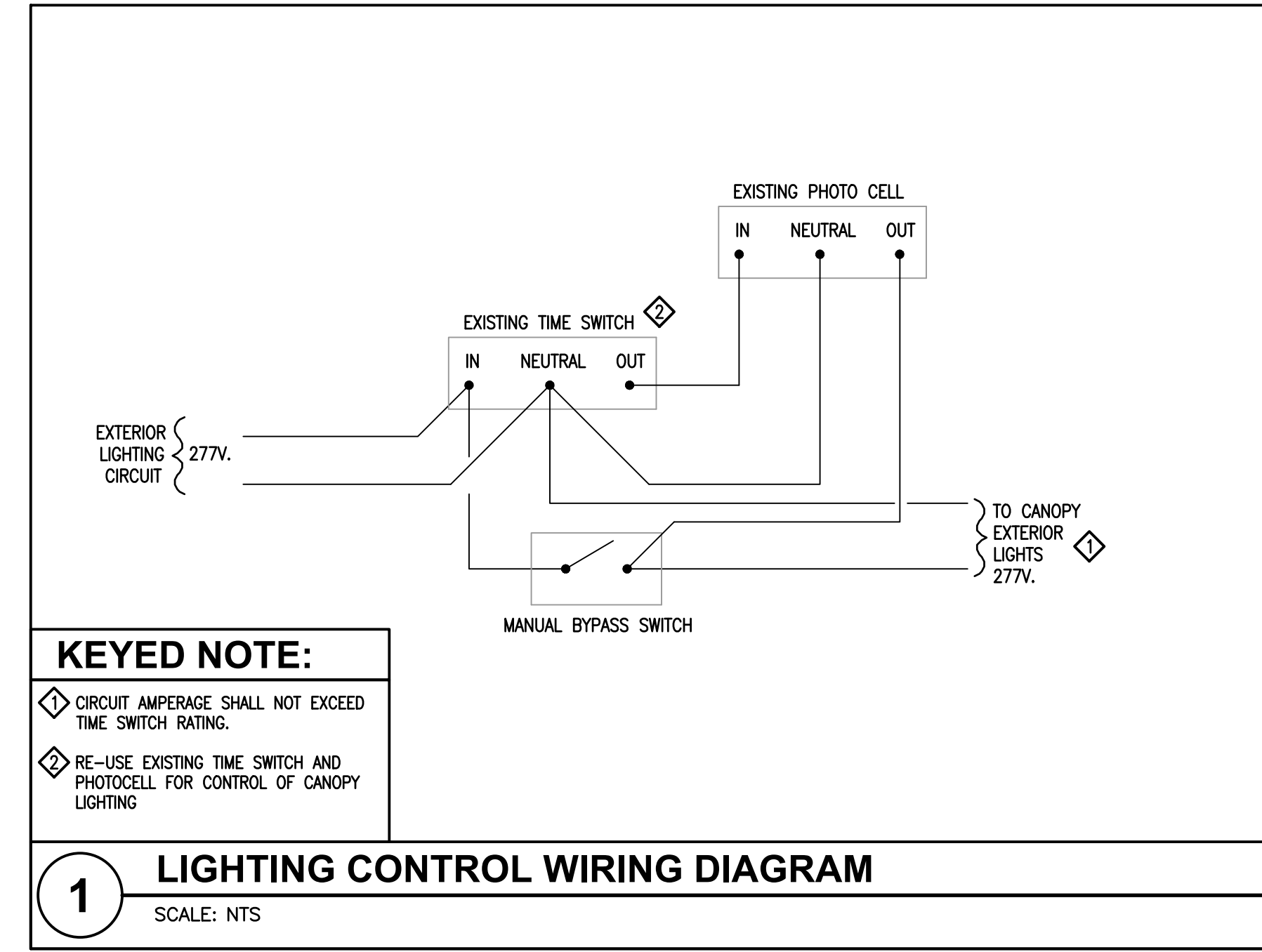
ELECTRICAL: 1837 S. EAST BAY BLVD. PROVO, UTAH 84606  
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Mechanical: 1837 S. EAST BAY BLVD. PROVO, UTAH 84606  
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LIGHT FIXTURE SCHEDULE										
FIXTURE NUMBER	FIXTURE MANUFACTURER	FIXTURE CATALOG #	LAMPS		FIXTURE			DESCRIPTION	REMARKS	
			TYPE	QTY.	VOLTS	WATTS	MOUNTING			
F1	CREE	CPY250-A-DM-F-B-UL-WH-DIM	LED	-	UNV	122	SURFACE CEILING	LED LOW PROFILE CANOPY LUMINAIRE	FIXTURES INCLUDED WITH CANOPY KIT AND INSTALLED BY CANOPY INSTALLER	



**EXISTING PANEL SCHEDULE "HA"**

VOLTAGE: 480 V/ 277 VOLTS      BUS RATING (AMPS): 400      REMARKS:

MOUNTING: SURFACE      PHASE: 3      MAIN LUGS ONLY

ENCLOSURE: NEMA 1      WIRE: 4      MINIMUM EQUIPMENT RATING: 22,000      AMPS (RMS-SYM)

No.	AMPS	POLE	MOD.	CIRCUIT NAME	FEEDER			LOAD/PHASE (VA)			FEEDER			CIRCUIT NAME	CIRCUIT BREAKER			
					C	WIRE	GRD	WATTS	ØA	ØB	ØC	WATTS	ØA		ØB	ØC	MOD.	POLE
1	20	1	EXST	LIGHTING PROCESSING (A)	-	-	-	1.00	2,310	4,970	-	-	-	LIGHTING PROCESSING (B)	EXST	1	20	2
3	20	1	EXST	LIGHTING PROCESSING (D)	-	-	-	1.00	2,310	-	4,970	-	-	LIGHTING PROCESSING (F)	EXST	1	20	4
5	20	1	EXST	LIGHTING PROCESSING (E)	-	-	-	1.00	2,310	-	-	4,970	-	LIGHTING PROCESSING (G)	EXST	1	20	6
7	20	1	EXST	LIGHTING OFFICES/FAMILY SERV.	-	-	-	1.00	3,220	4,830	-	-	-	LIGHTING PROCESSING (H)	EXST	1	20	8
9	20	1	EXST	LIGHTING OFFICES/FAMILY SERV.	-	-	-	1.00	3,450	-	-	6,670	-	LIGHTING PROCESSING (I)	EXST	1	20	10
11	20	1	EXST	SIGNS OUTSIDE BUILDING	-	-	-	1.00	770	-	-	2,900	2,130	BUILDING SECURITY LIGHTING	EXST	1	20	12
13	30	1	EXST	PARKING LIGHTING	-	-	-	1.00	3,300	4,300	-	-	-	DROP OFF CANOPY LIGHTING	EXST	1	20	14
15	20	1	EXST	PARKING LIGHTING	-	-	-	1.00	2,100	-	4,500	-	-	AS-IS CANOPY LIGHTING	EXST	1	20	16
17	20	1	EXST	PARKING LIGHTING	-	-	-	1.00	1,800	-	-	4,200	2,400	AS-IS CANOPY LIGHTING	EXST	1	20	18
19	20	1	EXST	PARKING LIGHTING	-	-	-	1.00	2,700	2,700	-	-	-	SIGN	EXST	1	20	20
21	20	1	EXST	PARKING LOT SPOTS	-	-	-	1.00	-	0	-	-	-	SIGN	EXST	1	20	22
23	20	1	NEW	NEW CANOPY LIGHTING	1"	#10	#10	1.25	1,830	-	1,830	-	-	SIGN	EXST	1	20	24
25	20	3	EXST	RTU-11 EMPLOYMENT LOBBY	-	-	-	1.00	2,640	-	-	5,900	-	RTU-8 FM OFFICES	EXST	3	25	26
27	-	-	-	-	-	-	-	1.00	2,640	-	-	5,900	-	-	-	-	-	28
29	-	-	-	-	-	-	-	1.00	2,640	-	-	5,900	-	-	-	-	-	30
31	60	3	EXST	TRASH COMPACTOR	-	-	-	1.00	2,640	4,640	-	-	-	RTU-12 EMPLOYMENT OFFICE	EXST	3	20	32
33	-	-	-	-	-	-	-	1.00	7,500	-	-	9,500	-	-	-	-	-	34
35	-	-	-	-	-	-	-	1.00	7,500	-	-	9,500	-	-	-	-	-	36
37	60	3	EXST	RTU-15 RETAIL	-	-	-	1.00	9,090	18,180	-	-	-	RTU-14 RETAIL	EXST	3	60	38
39	-	-	-	-	-	-	-	1.00	9,090	-	-	9,090	-	-	-	-	-	40
41	-	-	-	-	-	-	-	1.00	9,090	-	-	18,180	-	-	-	-	-	42

**NOTES:**

- ALL INSULATION ON CONDUCTORS TO BE THHN UNLESS NOTED OTHERWISE. INSULATION ON ALL UNDERGROUND EXTERIOR CONDUCTORS SHALL BE THHW.
- LOAD DEMANDS CALCULATED AS PER SECTIONS 210 & 220 OF THE NATIONAL ELECTRICAL CODE.
- PANEL COVER SHALL BE FIELD MARKED FOR FLASH PROTECTION WITH A PERMANENT LABEL AS REQUIRED BY THE NATIONAL ELECTRICAL CODE SECTION 110. LABEL SHALL READ: "DANGER: POTENTIAL ARC FLASH HAZARD"
- ABBREVIATIONS: CO-CONVENIENCE OUTLET, RR-RESTROOM, (N)ORTH, (S)OUTH, (E)AST, (W)EST.

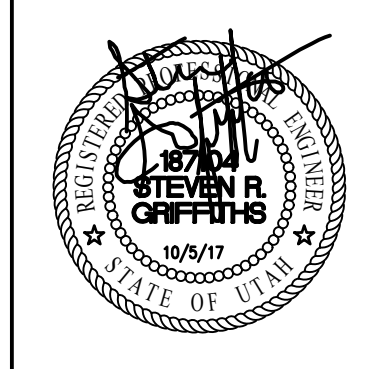
ØA	ØB	ØC	TOTALS	CONNECTED LOAD (VA)
45,520	49,720	47,480	142,720	CONNECTED LOAD (A)
0	0	458	458	DEMAND FACTOR ADJUSTMENTS (VA)
45,520	49,720	47,938	143,178	TOTAL LOAD (VA)
164	179	173	516	TOTAL LOAD (A)
32%	35%	33%	179	PHASE BALANCE

**4 TYPICAL CONDUIT RACK DETAIL**  
SCALE: NTS

**ROYAL ENGINEERING**

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SHEET TITLE:  
ELECTRICAL  
DETAILS AND  
SCHEDULES

# DESERET INDUSTRIES COVER

## AMERICAN FORK, UTAH

### PRECISION CANOPY

#### GENERAL STRUCTURAL NOTES

##### 1. 2015 International Building Code Design Criteria (ASCE 7-10)

- A. Floor Live Load . . . . . N.A.
- B. Roof Live Load . . . . . 20 psf
- C. Roof Snow Load Data
  - Ground Snow Load  $P_g$  . . . . . 43 psf
  - Flat-roof Snow Load  $P_f$  . . . . . 36 psf
  - Exposure Factor  $C_e$  . . . . . 1.0
  - Importance Factor  $I_s$  . . . . . 1.0
  - Thermal Factor  $C_t$  . . . . . 1.2
- D. Wind Design Data
  - Ultimate Design Wind Speed  $V_{ult}$  . . . . . 115 mph
  - Nominal Design Wind Speed  $V_{asd}$  . . . . . 90 mph
  - Risk Category . . . . . II
  - Exposure . . . . . C
  - Component & Cladding . . . . . See ASCE 7-10 Chapter 30
- E. Earthquake Design Data
  - Risk Category . . . . . II
  - Importance Factor  $I_e$  . . . . . 1.0
  - Mapped Spectral Parameters
    - $S_s$  . . . . . 1.204g
    - $S_1$  . . . . . 0.417g
  - Site Class . . . . . D
  - Design Spectral Parameters
    - $S_{DS}$  . . . . . 0.817g
    - $S_{D1}$  . . . . . 0.440g
  - Seismic Design Category . . . . . C
  - Seismic Force Resisting System . . . . . OSMF, OSCBF
  - Seismic Response Coefficient  $C_s$  . . . . . 0.233W, 0.251W
  - Response Modification Factor  $R$  . . . . . 3.5, 3.25
  - Analysis Procedure . . . . . Equivalent Lateral Force Procedure
- F. Frost Depth . . . . . 2'-6"

##### 2. Earthwork

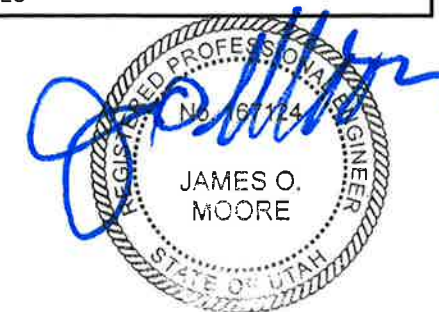
- A. Foundation Design Values (assumed)
  - i. Allowable Soil Bearing Pressure - 1500 psf
  - ii. Coefficient of Friction - 0.25
  - iii. Passive Earth Pressure - 150 psf/ft of depth
- B. The building pad area shall be stripped of all frozen soil, debris, vegetation, and topsoil. All fill soils and any remaining loose natural soils shall be excavated to expose suitable natural soils.
- C. Proof roll the entire building pad area to locate and remove all soft spots. Replace with compacted structural fill.
- D. Place all footings and slabs on undisturbed natural soil or on properly compacted structural fill. Contractor shall verify that soil under footings is suitable to support footings.
- E. Structural Fill: Structural fill should consist of well-graded sandy gravels with a maximum particle size of 3 inches and 5 to 15 percent fines (materials passing the No. 200 sieve). The liquid limit of fines should not exceed 35 and the plasticity index should be below 15. All fill soils should be free from topsoils, highly organic material, frozen soil, and other deleterious materials. Structural fill should be placed in maximum 8-inch thick loose lifts at a moisture content within 2 percent of optimum and compacted to at least 95 percent of modified proctor density (ASTM D1557) under the building and 90 percent under concrete flatwork.
- F. It is the responsibility of the contractor to ensure that the depth of the bottom of the foundation is far enough below the adjacent grade to ensure adequate frost protection.

##### 3. Concrete and Reinforcement

- A. Material Standards
  - i. Concrete
    - a. Footings: Exposure Classes F1, S0, W0, C1  
 $f'_c = 3500$  p.s.i., max. w/cm ratio = 0.55
    - b. Exterior Walls: Exposure Classes F1, S0, W0, C1  
 $f'_c = 3500$  p.s.i., max. w/cm ratio = 0.55
    - c. Interior Slabs: Exposure Classes F0, S0, W0, C0  
 $f'_c = 3500$  p.s.i., max. w/cm ratio = 0.55
    - d. Air content for Exposures F1-F3 must meet the requirements of Table 19.3.3.1 of ACI 318-14. Air-entraining admixtures shall conform to ASTM C260
    - e. The cement type for Exposures S1-S3 must meet the requirements of Table 19.3.2.1 of ACI 318-14. Cement shall conform to ASTM C150
    - f. Calcium Chloride admixture shall not be used in Exposures S2 and S3
    - g. Normal weight aggregates - ASTM C33
    - h. Cold weather concrete shall conform to ASTM C94, ACI 306R, & ACI 301.
      - 1. Adequate equipment shall be provided for heating concrete materials and protecting concrete during freezing or near-freezing weather.
      - 2. Frozen materials or materials containing ice shall not be used.
      - 3. Forms, fillers, and ground with which concrete is to come in contact shall be free from frost and ice.
      - 4. Concrete materials and production methods shall be selected so that the concrete temperature at delivery complies with the specified temperature limits.
    - j. Warm weather concrete shall conform to ACI 305R, ACI 301, ACI 305.1.
      - 1. The maximum concrete temperature at the time of placement is 95°.
      - 2. Concrete materials and production methods shall be selected so that the concrete temperature at delivery complies with the specified temperature limits.
      - 3. Handling, placing, protection, and curing procedures shall limit concrete temperatures or water evaporation that could reduce strength, serviceability, and durability of the member of structure.
  - ii. Reinforcing
    - a. Rebar - ASTM A615 Grade 60 ( $F_y = 60$  ksi)
    - b. Welded wire - ASTM A1064
    - c. Epoxy - Simpson SET-XP (ICC-ES ESR-2508) or Hilti HIT-RE 500-SD (ICC-ES ESR-2322)
  - iii. Anchor Rods/Bolts
    - a. Steel column anchor rods/bolts - ASTM F1554 Grade 36 with ASTM A563 heavy hex nuts and hardened washers
    - b. Adhesive (epoxy) anchors - Simpson SET-XP (ICC-ES ESR-2508) or Hilti HIT-RE 500-SD (ICC-ES ESR-2322)
    - c. Expansion anchors - Simpson Strong-Bolt (ICC-ES ESR-1771) or Hilti Kwik Bolt TZ (ICC-ES ESR-1917).
    - d. Screw Anchors - Simpson Titen HD (ICC-ES ESR-2713), Hilti Kwik HUS-TZ (ICC-ES ESR-3027), or Powers Fasteners Wedge-Bolt+ (ICC-ES ESR-2526)
    - e. Use of hooked anchor rods/bolts is limited under the ACI and the IBC. Headed anchor rods/bolts must be used where indicated in the details.
    - f. The symbols  $\odot$  A.R./ $\odot$  A.B. as shown in the drawings indicate the center line of the anchor rod/bolt pattern, not the center line of any individual anchor rod/bolt.

- B. Detail reinforcing to comply with ACI 315 "Manual of Standard Practice for Detailing Reinforcing Concrete Structures" and the Concrete Reinforcing Steel Institute (CRSI) recommendations.
  - i. Minimum clear concrete cover for reinforcement shall be as follows unless noted otherwise:
    - a. Concrete cast directly against and permanently exposed to earth - 3"
    - b. Concrete exposed to weather or earth:
      - 1. #5 bars or smaller - 1 1/2"
      - 2. #6 bars or larger - 2"
    - c. Concrete not exposed to weather or in contact with the ground - 3/4"
    - d. Slabs on grade - as shown in details, 3/4" min. from top of slabs not exposed to weather
  - ii. Lap Splice Lengths (unless noted otherwise)
    - a.  $f'_c = 2500-3500$  p.s.i.
      - 1. #6 and smaller - 36 bar diameters
      - 2. #7 and larger - 45 bar diameters
    - b.  $f'_c = 4000$  p.s.i. or greater
      - 1. #6 and smaller - 29 bar diameters
      - 2. #7 and larger - 36 bar diameters
    - c. Lap splice lengths may be decreased by 25% for slabs on grade and horizontal wall reinforcing.
    - d. Increase lap splice lengths by 50% where epoxy coated bars are used.

DRAWING	INDEX OF DRAWINGS
GN1	GENERAL STRUCTURAL NOTES
GN2	GENERAL STRUCTURAL NOTES
F-1	FOUNDATION & ANCHOR BOLT PLAN
F-2	FOUNDATION DETAILS
S-1	ROOF FRAMING PLAN
S-2	SIDEWALL FRAMING
S-3	SIDEWALL FRAMING
S-4	RIGID FRAME ELEVATION
S-5	RIGID FRAME ELEVATION
S-6	ERECTION DETAILS
S-7	ERECTION DETAILS



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Structural Engineering Consulting

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SHEET TITLE: GENERAL STRUCTURAL NOTES

JOB NAME: DESERET INDUSTRIES COVER

LOCATION: AMERICAN FORK, UTAH

CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	FOR PERMIT

SHEET NUMBER:

# GN1

DRAWN BY: J.J.
ENGINEER: J.O. MOORE
AVE JOB NUMBER: 170669



- iii. Stagger splices in walls so that no two adjacent bars are spliced in the same location, unless shown otherwise.
  - iv. Make all bars continuous around corners or provide corner bars of equal size and spacing.
  - v. Vertical bars in walls, grade beams, and piers to terminate in footings with ACI standard hooks (12 bar diameters) to within 4" of the bottom of the footing unless noted otherwise.
  - vi. Horizontal wall reinforcing shall terminate at the ends of walls with a 90 degree hook plus a 6 bar diameter extension, unless shown otherwise.
  - vii. Horizontal wall reinforcing shall be continuous through construction and control joints.
  - viii. Splices in horizontal reinforcement shall be staggered. Splices in two curtains (where used) shall not occur in the same location.
  - ix. Use chairs or other support devices as required for proper clearance.
  - x. Rebar hairpins shall be centered in slabs and shall be wire tied to the slab reinforcing (if any). Rebar hairpins shall be continuous through walls and piers; lap splices in hairpins may only occur in the floor slab unless noted otherwise.
  - xi. Unless noted otherwise, openings in walls shall be reinforced with #5 bar on all sides of the opening. Reinforcing shall extend 24" min. past the edge of the opening. For one layer of wall reinforcing provide (1) #5 bar around openings, for two layers provide (2) #5 bars.
- C. Control joints in slabs on grade are recommended to control cracking. See plans for control joint spacing and details.
- D. Slabs and grade beams shall not have joints in a horizontal plane. All reinforcement shall be continuous through all construction joints.
- E. Floor slab thickness and reinforcing shown in these drawings are adequate to support typical uniform loads only. Mountain View Engineering has not designed the slab for any specific concentrated forces such as those from vehicles, storage racks, or heavy equipment (unless noted otherwise).
- F. Welding of rebar is not allowed unless specifically indicated in the drawings. All embedments, reinforcing, and dowels shall be securely tied to framework or to adjacent reinforcing prior to placement of the concrete. Tack welding of rebar joints in grade beams, walls, or cages is not allowed. Where welding of rebar is shown in the drawings, all rebar to be welded shall be ASTM A706 Grade 60.

#### 4. Structural Steel

- A. Material:
- i. Angles, Plates, and Channels: ASTM A36 (Fy = 36 k.s.i.)
  - ii. Wide Flanges: ASTM A992 (Fy = 50 k.s.i.)
  - iii. Tubes (HSS): ASTM A500, Grade B (Fy = 46 k.s.i.)
  - iv. Pipes: ASTM A53, Grade B (Fy = 35 k.s.i.)
- B. Fabrication and construction shall comply with the latest edition of the following Codes and Standards:
- i. American Institute of Steel Construction (AISC) "Specification for Structural Steel Buildings." (360-10)
  - ii. AISC "Seismic Provisions for Structural Steel Buildings," including supplement No. 1 (341-10)
  - iii. AISC "Code of Standard Practice."
  - iv. RCSC "Specification for Structural Joints Using ASTM A325 or A490 Bolts."
  - v. Steel Joist Institute (SJI), "Standard Specifications and Code of Standard Practice."
  - vi. American Welding Society (AWS), Structural Welding Code (specific items do not apply when they conflict with the AISC requirements).
  - vii. American Iron and Steel Institute (AISI), "Specification for the Design of Cold-Formed Steel Structural Members".
- C. Welding:
- i. Certification of Welders: All shop and field welding shall be executed by AWS certified welders. Certification shall be considered current if dated within the past 12 months. Welders will be considered certified if they have been certified by AWS and their work records are current within every six month period thereafter as required by AWS. Certification and records must comply with AWS Standards. Certification and appropriate records must be provided to the inspector prior to beginning work.
  - ii. Electrodes: E-70XX or as noted otherwise. E-60XX may be used for welding steel floor and roof decks.
  - iii. Minimum Welds: All intersecting steel shapes which are not bolted shall be connected by a fillet weld all around, unless noted otherwise.
  - iv. Welded reinforcing bars shall be ASTM A706 Grade 60.

- D. Bolted Connections:
- i. Use ASTM A325N bolts for hot-rolled steel to steel (e.g. girder to column, rafter to column/cap plate, beam to beam, etc.) connections unless noted otherwise in the drawings. All connections shall conform to the RCSC "Specification for Structural Joints Using ASTM A325 or A490 Bolts".
  - ii. A325 bolts shall be pretensioned using an acceptable method such as "Turn of the Nut" as per the RCSC Specification Section 8.2 at all 8-bolt wide flange to wide flange ridge and haunch connections only.
  - iii. All other A325 bolts shall be fully tightened to the "Snug Tight" condition in accordance with the RCSC Specification Section 8.1.
- E. Provide full height web stiffener plates to each side of all beams bearing on top of columns. Plate thickness shall match the thickness of the beam web except that the thickness need not exceed 1/4" unless noted otherwise on drawings.
- F. All structural steel, except plates embedded in concrete or masonry, to have one coat (min.) of gray shop primer, 1.5 mil minimum thickness.
- G. Open Web Steel Joists and Girders:
- i. The steel joist supplier shall be responsible for the design of all parallel chord and double pitched top chord steel joists and girders.
  - ii. Field Modifications: Do not modify any joist or girder, including holes through the top and bottom chords, without the written consent and direction from the manufacturer.
  - iii. At completion of fabrication, the steel joist manufacturer shall submit a certificate of compliance as an approved fabricator and that the work was performed in accordance with approved construction documents and with SJI standard specifications (IBC 2206.5).
- H. Cold Formed Girts & Purlins:
- i. All cold formed cees, zeas, and eave struts shall be MBCI standard or equal.
  - ii. All girt and purlin connection bolts shall be GR-5 or equal.
- J. Use 26 gage roof and wall panels with trims as required.
- K. Flange braces as shown by marks UB and FB to be 2"x2"x1/8" angle.
- L. Steel Stairs, Handrails, and Guardrails
- i. Design of steel stairs, handrails, and guardrails is not by Mountain View Engineering.
  - ii. All stairs, handrails, and guardrails shall comply with the requirements of the 2015 IBC unless noted otherwise in the project specifications.
  - iii. The fabricator shall be responsible for the design and certification of all steel stairs, handrails, and guardrails, including member sizes and connection details.
  - iv. See the architectural plans for all stair information including, but not limited to, stair layout, dimensions, and style.

#### 5. Special Inspections

- A. Special inspections, as required by Section 1705 of the IBC, shall be provided by an independent agency employed by the owner unless waived by the building official. The contractor shall coordinate and cooperate with the required inspections. Items requiring special inspection are:
- i. Steel Construction (AISC 360-10 Chapter N)
    - a. Field welding (if any is used).
    - b. High-strength bolts.
    - c. Structural steel shall be fabricated by a fabricator that has been approved by the local building department or shall have special inspection as per IBC 1705.2.1 performed by an approved inspection agency.
    - d. Open web joists and girders (IBC Table 1705.2.3.)
  - ii. Concrete Construction (IBC 1705.3)
    - a. Special inspections of concrete footings, grade beams, walls, and slabs are not required as per Exceptions 1, 2.3, 3, 4, & 5 to IBC Section 1705.3. Third party special inspection of reinforcing placement need only be performed where specifically required by the building official.
    - b. Special inspection of anchor rods/bolts is required per IBC Table 1705.3. Special inspection may be waived subject to the approval of the building official.
    - c. Special inspection of rebar welding is required (if any is used).
- B. Special inspector must be qualified and approved by local building department.

#### 6. Miscellaneous

- A. The project specifications are not superseded by the General Structural Notes. Notes and details on the drawings shall take precedence over General Structural Notes and typical details. Should any of the detailed instructions shown on the plans conflict with the General Structural Notes, or with each other, the strictest provisions shall govern.
- B. It is solely the responsibility of each contractor to follow all applicable safety codes and regulations during all phases of construction. The engineer is not engaged in, and does not supervise, construction.
- C. Erection, Shoring, and Bracing
- i. It is the contractor's responsibility to determine erection procedures and sequence, and to ensure the stability of the building and its component parts during erection.
  - ii. It is solely the contractor's responsibility to provide any temporary shoring, bracing, guys, and tie downs that may be necessary to provide adequate vertical and lateral support. Such material is not shown on the drawings. Shoring and bracing shall remain in place until all permanent members are in place and all final connections are completed, including all roof and floor attachments.
  - iii. The building shall not be considered stable until all connections are complete.
  - iv. The engineer has no expertise in, and takes no responsibility for, construction means and methods or job site safety during construction. Approval of submittals made by the contractor which may contain information related to construction methods or safety issues, or participation in meetings where such issues might be discussed, shall not be construed as voluntary assumption by the engineer of any responsibility for safety procedures.
- D. Equipment framing loads, openings, and structure in any way related to mechanical, plumbing, or electrical requirements are shown for bidding purposes only. The contractor shall coordinate this information with the involved trades before proceeding with such portion of the work. Excess cost related to variation in these requirements shall be borne by the appropriate contractor.
- E. The contractor shall notify engineer of any variations in dimensions.
- F. The engineer is not responsible for any deviations from these plans unless such changes are authorized in writing by the engineer.



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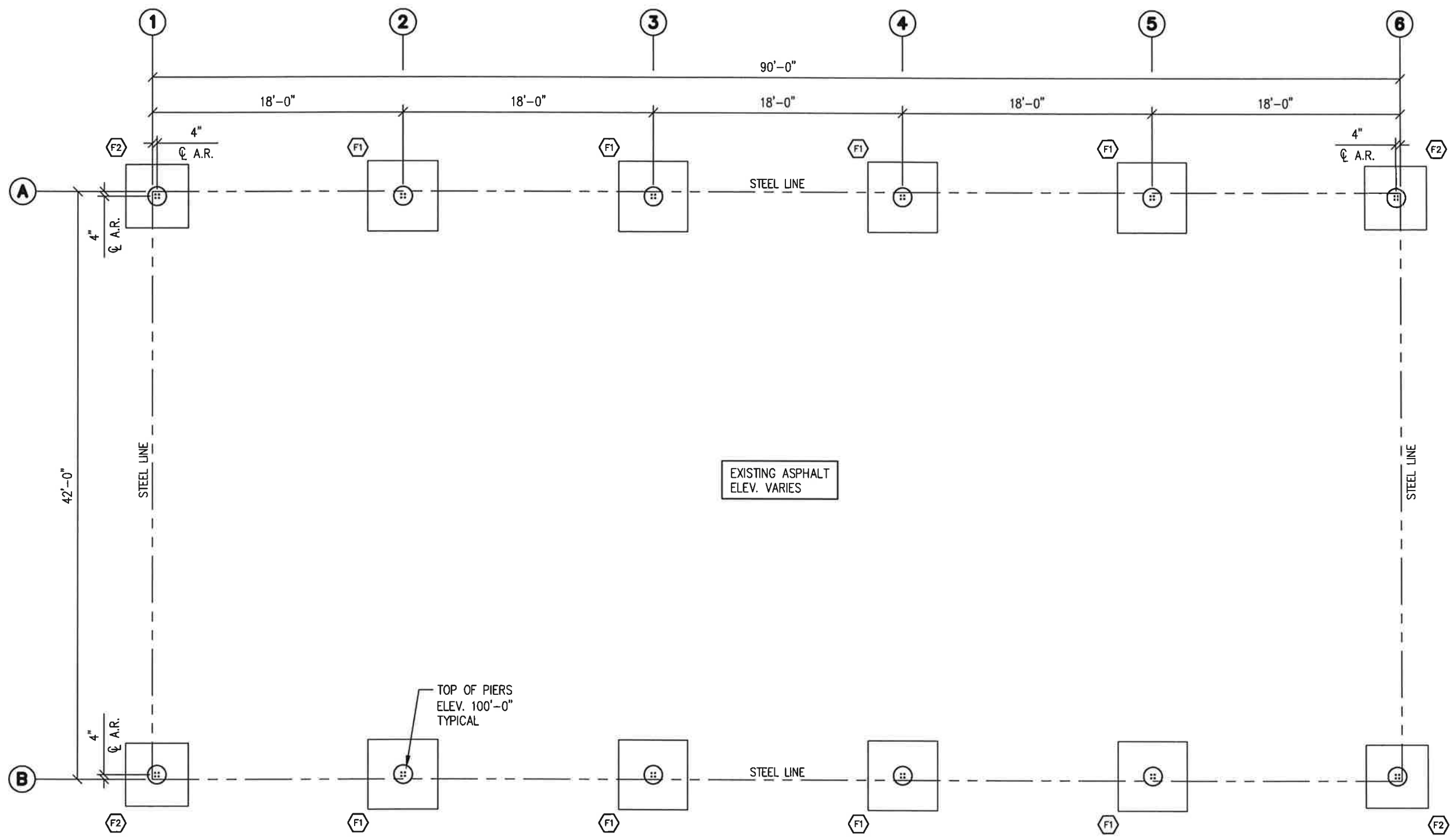
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SHEET TITLE: GENERAL STRUCTURAL NOTES  
JOB NAME: DESERET INDUSTRIES COVER  
LOCATION: AMERICAN FORK, UTAH  
CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	FOR PERMIT

SHEET NUMBER:  
**GN2**

DRAWN BY: J.J.  
ENGINEER: J.O. MOORE  
MVE JOB NUMBER: 170669



F1 INDICATES APPLICABLE FOOTING DETAIL.

**FOUNDATION AND ANCHOR ROD PLAN**  
SCALE: 1/8" = 1'-0"



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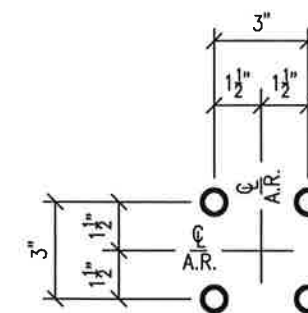
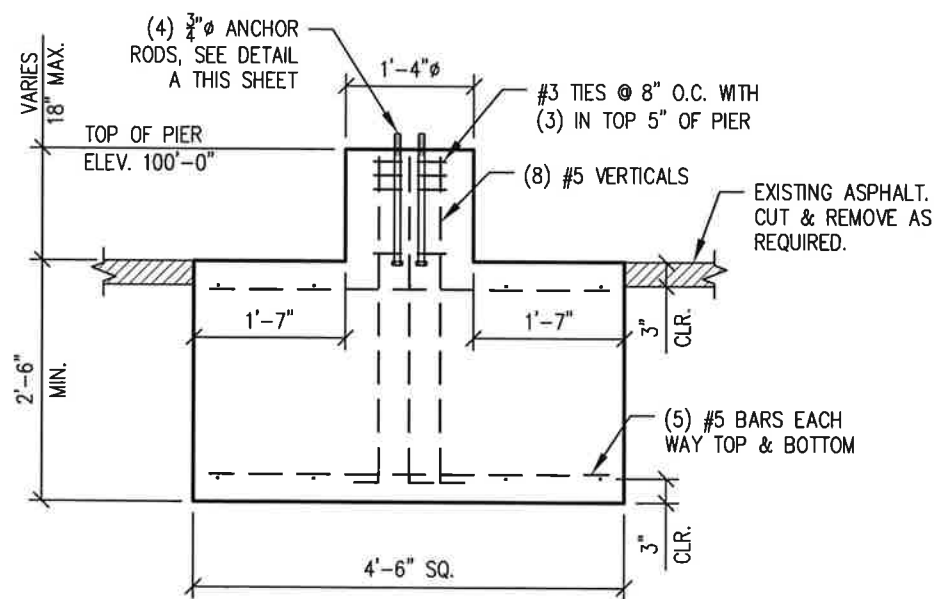
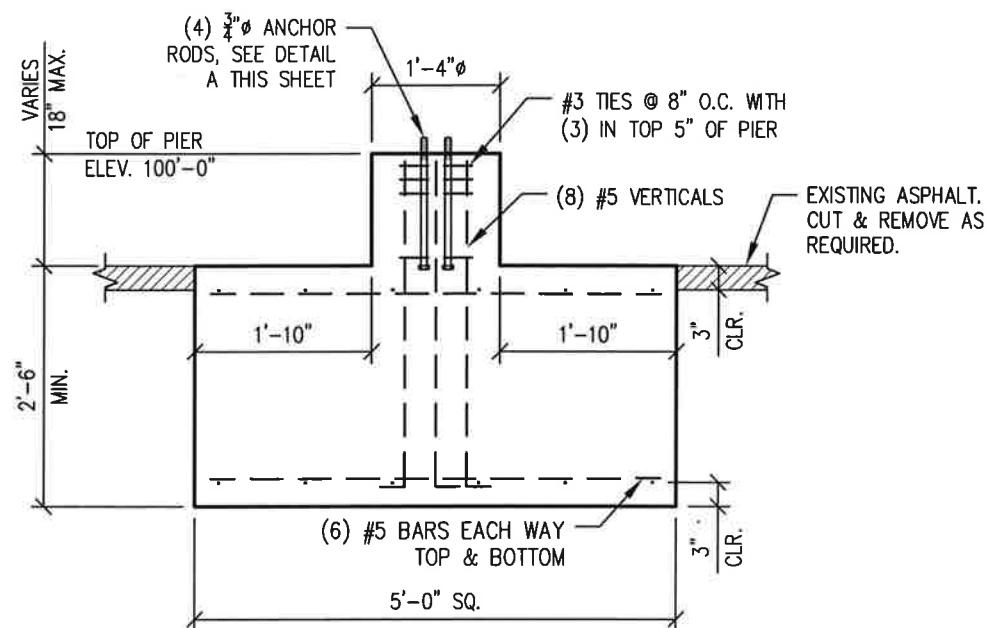
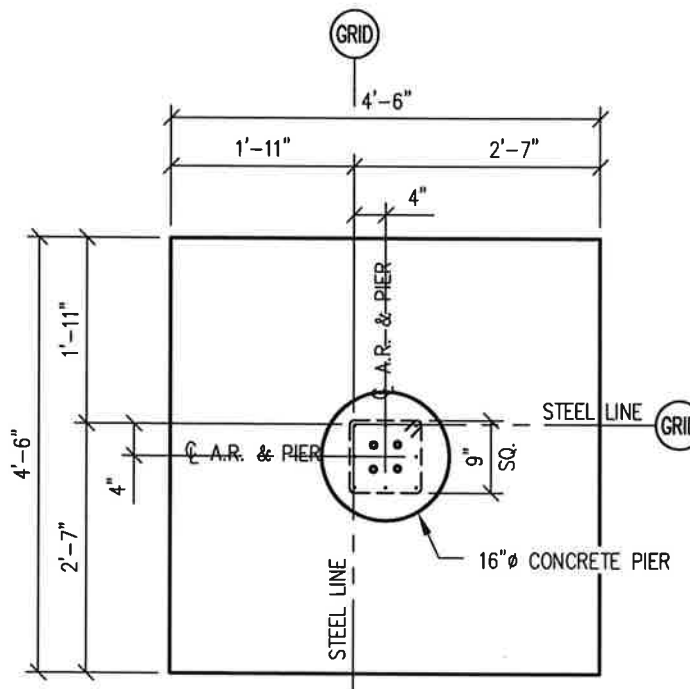
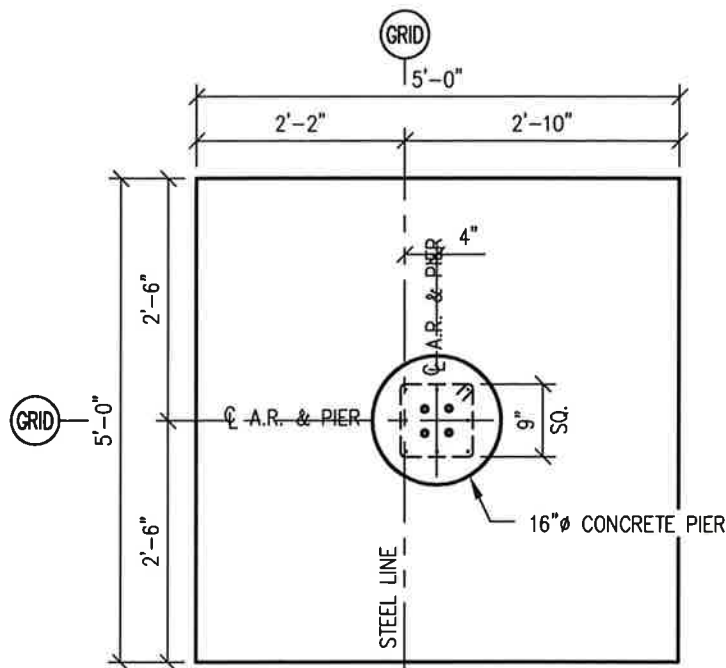
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 JOB NAME: DESERET INDUSTRIES COVER  
 LOCATION: AMERICAN FORK, UTAH  
 CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	BY:	DESCRIPTION:
DATE:	J.J.	FOR PERMIT
10-23-17		

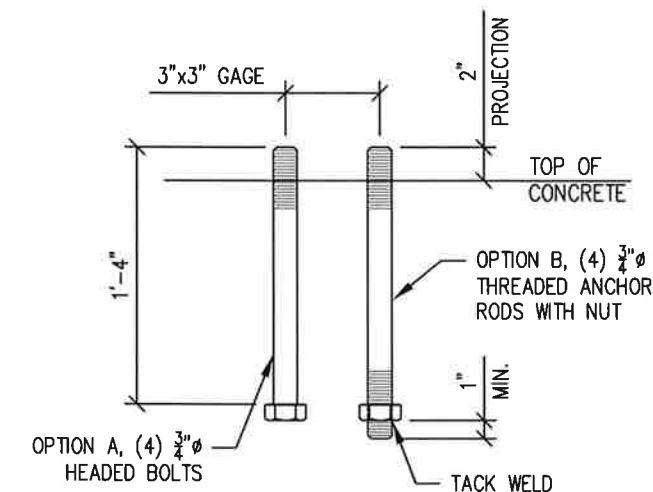
SHEET NUMBER:  
**F-1**

DRAWN BY: J.J.  
 ENGINEER: J.O. MOORE  
 MVE JOB NUMBER: 170669

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TOP VIEW



**A** ANCHOR BOLT DETAIL  
1/2"=1'-0"



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**F1** INTERIOR FRAMES FOOTING DETAIL  
1/2"=1'-0" @ LINES 2 THRU 5

**F2** END FRAMES FOOTING DETAIL  
1/2"=1'-0" @ LINES 1 AND 6



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SHEET TITLE: FOUNDATION DETAILS

JOB NAME: DESERET INDUSTRIES COVER

LOCATION: AMERICAN FORK, UTAH

CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES

DATE:	BY:	DESCRIPTION:
10-23-17	J.J.	FOR PERMIT

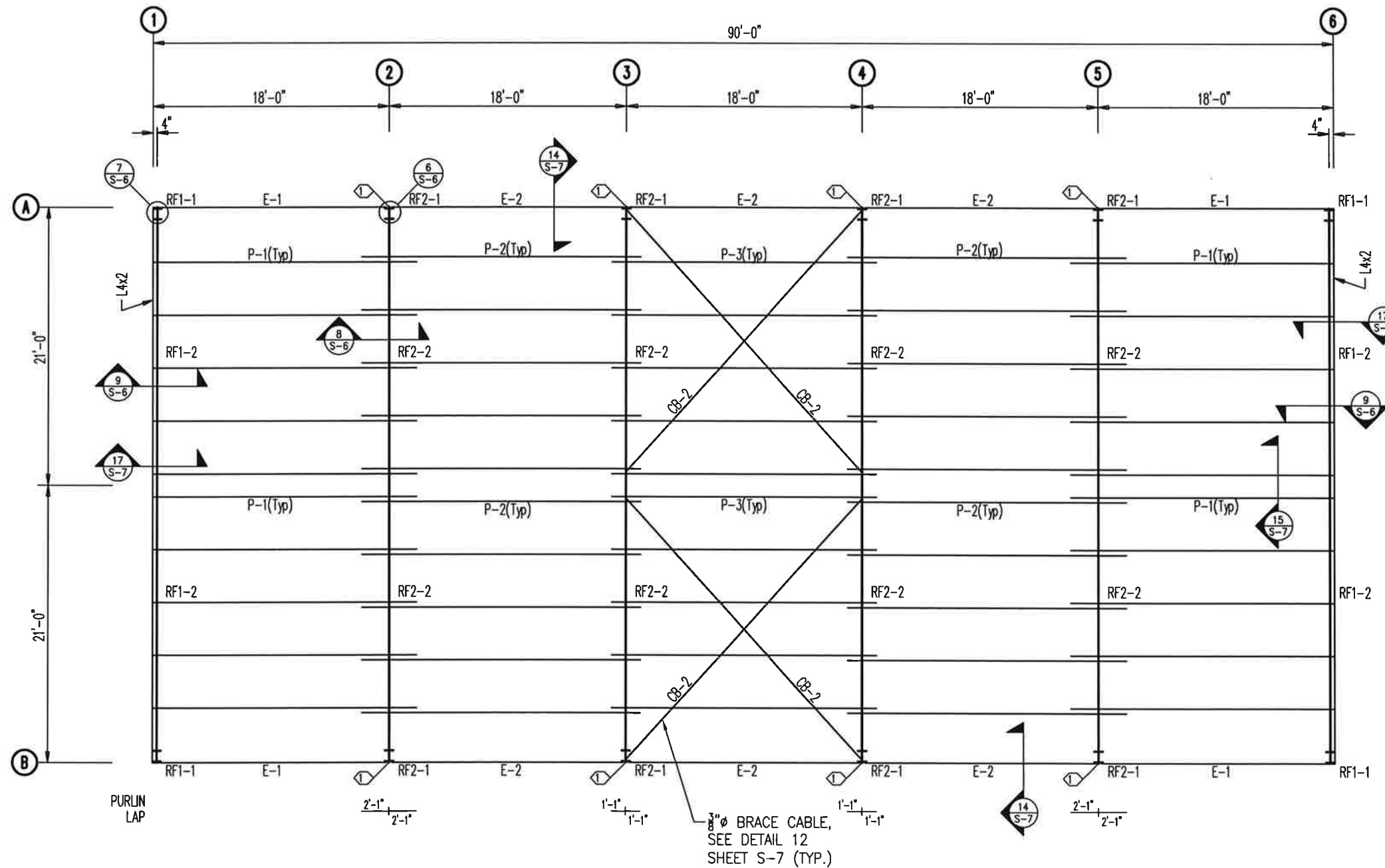
SHEET NUMBER:

**F-2**

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
MVE JOB NUMBER:	170669


SPECIAL BOLTS					
ROOF PLAN					
Ø ID	QUAN	TYPE	DIA	LENGTH	WASH
1	2	GR.5	1/2"	1 1/4"	2

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8X25Z14	20'-1"
P-2	8X25Z16	21'-2"
P-3	8X25Z16	20'-2"
E-1	8ES1L14	17'-11 3/4"
E-2	8ES1L14	17'-11 1/2"
CB-2	HW-375	26'-6 1/2"



ROOF FRAMING PLAN

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345 North Main Street Suite A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

SHEET TITLE: ROOF FRAMING PLAN  
 JOB NAME: DESERET INDUSTRIES COVER  
 LOCATION: AMERICAN FORK, UTAH  
 CONTRACTOR: PRECISION CANOPY

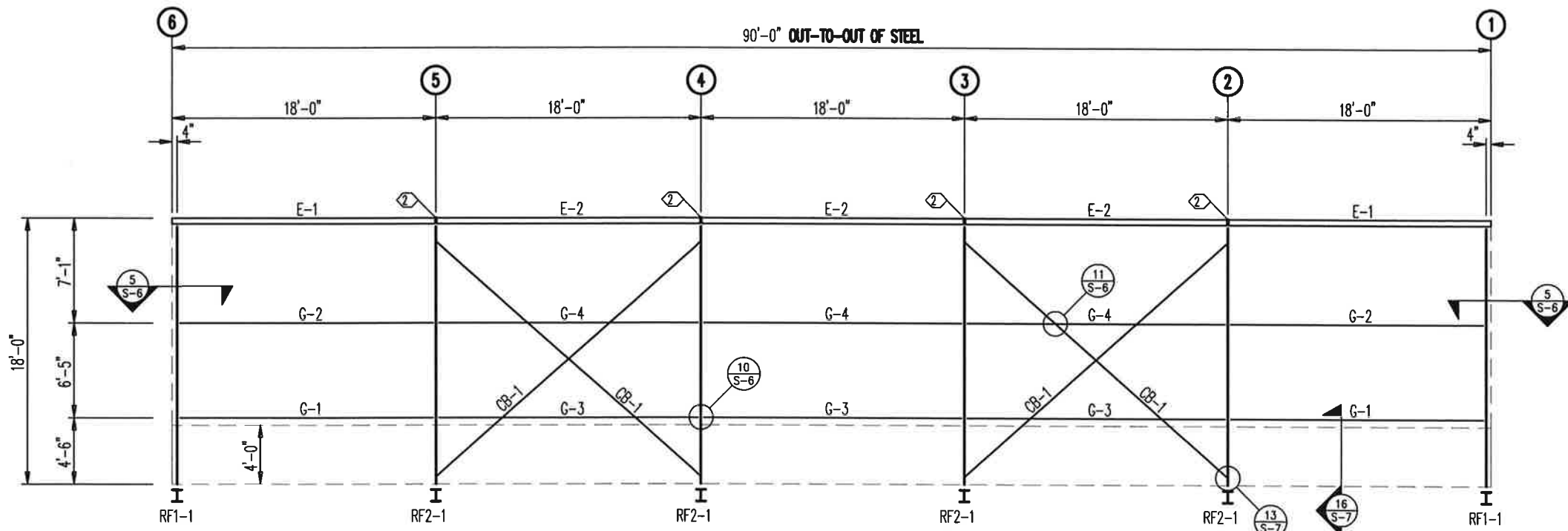
PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	J.J. FOR PERMIT



OCT 25 2017

SHEET NUMBER:  
**S-1**

DRAWN BY: J.J.  
 ENGINEER: J.O. MOORE  
 MVE JOB NUMBER: 170669

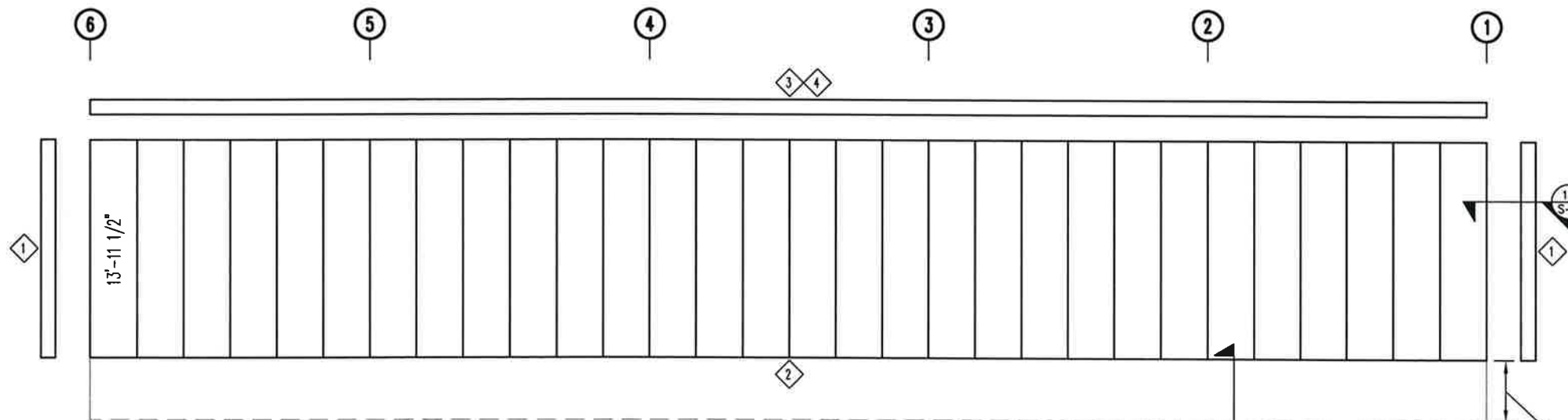


SIDEWALL FRAMING: FRAME LINE A

SPECIAL BOLTS					
○ ID	QUAN	TYPE	DIA	LENGTH	WASH
2	2	GR_5	1/2"	1 1/4"	2

MEMBER TABLE FRAME LINE A		
MARK	PART	LENGTH
E-1	8ES1L14	17'-11 3/4"
E-2	8ES1L14	17'-11 1/2"
G-1	8X25Z16	17'-0 3/4"
G-2	8X25Z14	17'-0 3/4"
G-3	8X25Z16	17'-4 7/16"
G-4	8X25Z14	17'-4 7/16"
CB-1	HW-375	24'-9 1/4"

TRIM TABLE FRAME LINE A		
◇ ID	PART	LENGTH
1	FL-23C	14'-2"
2	FL-871	20'-2"
3	FL-19	10'-2"
4	FL-19A	20'-2"



SIDEWALL SHEETING & TRIM: FRAME LINE A  
PANELS: 26 Ga. PR - NEED SIG 200



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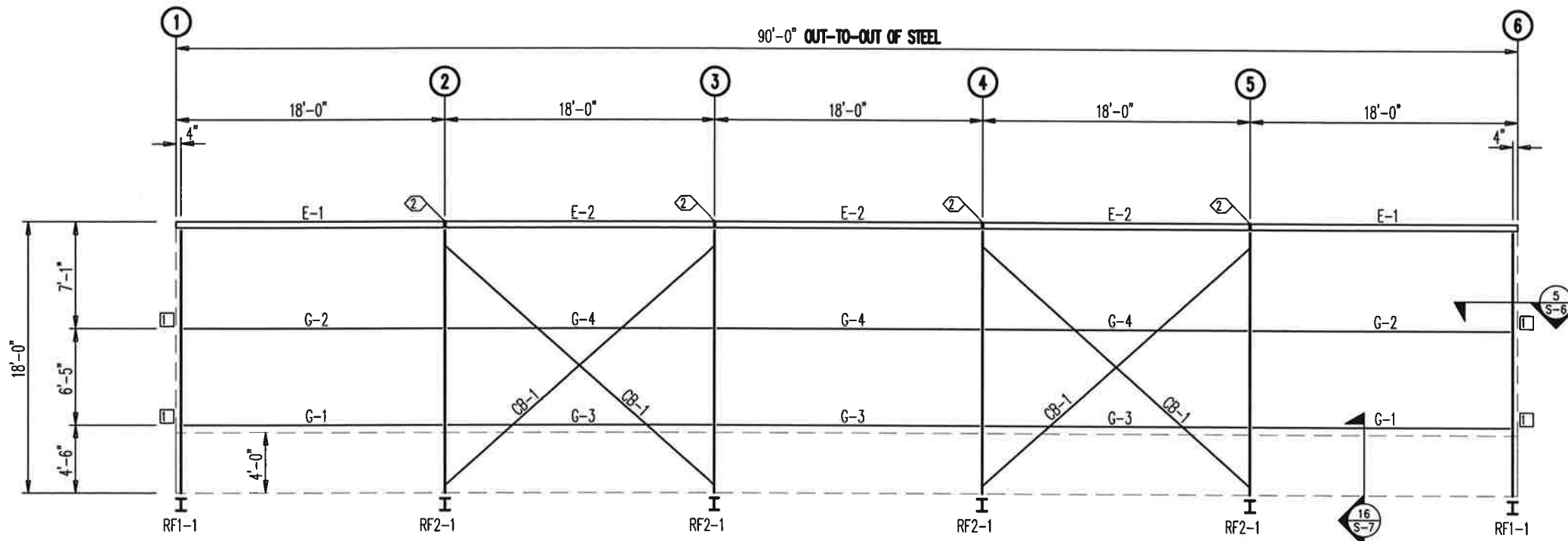
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SHEET TITLE: SIDEWALL FRAMING  
 JOB NAME: DESERET INDUSTRIES COVER  
 LOCATION: AMERICAN FORK, UTAH  
 CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	J.J. FOR PERMIT

SHEET NUMBER:  
**S-2**

DRAWN BY: J.J.  
 ENGINEER: J.O. MOORE  
 MVE JOB NUMBER: 170669



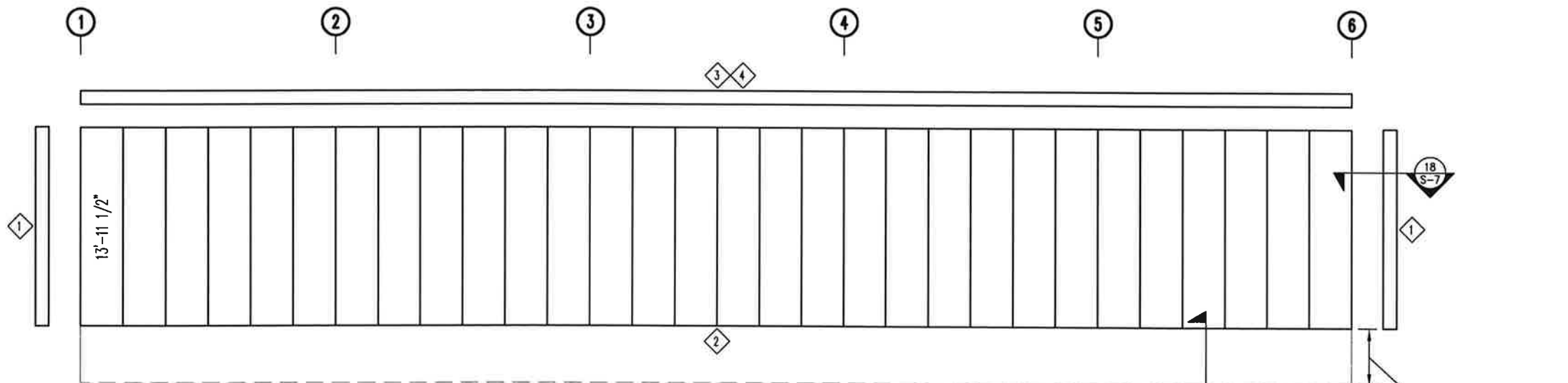
SIDEWALL FRAMING: FRAME LINE B

SPECIAL BOLTS					
Ø ID	QUAN	TYPE	DIA	LENGTH	WASH
2	2	GR. 5	1/2"	1 1/4"	2

MEMBER TABLE FRAME LINE B		
MARK	PART	LENGTH
E-1	BESIL14	17'-11 3/4"
E-2	BESIL14	17'-11 1/2"
G-1	8X25Z16	17'-0 3/4"
G-2	8X25Z14	17'-0 3/4"
G-3	8X25Z16	17'-4 7/16"
G-4	8X25Z14	17'-4 7/16"
CB-1	HW-375	24'-9 1/4"

TRIM TABLE FRAME LINE B		
Ø ID	PART	LENGTH
1	FL-23C	14'-2"
2	FL-871	20'-2"
3	FL-19	10'-2"
4	FL-19A	20'-2"

CONNECTION PLATES FRAME LINE B		
Ø ID	MARK/PART	
1	SC-5	



SIDEWALL SHEETING & TRIM: FRAME LINE B  
PANELS: 26 Ga. PR - NEED SIG 200

BOTTOM OF SHEETING IS 4'-0" ABOVE BASE OF COLUMN



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SHEET TITLE: SIDEWALL FRAMING  
 JOB NAME: DESERET INDUSTRIES COVER  
 LOCATION: AMERICAN FORK, UTAH  
 CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	J.J. FOR PERMIT

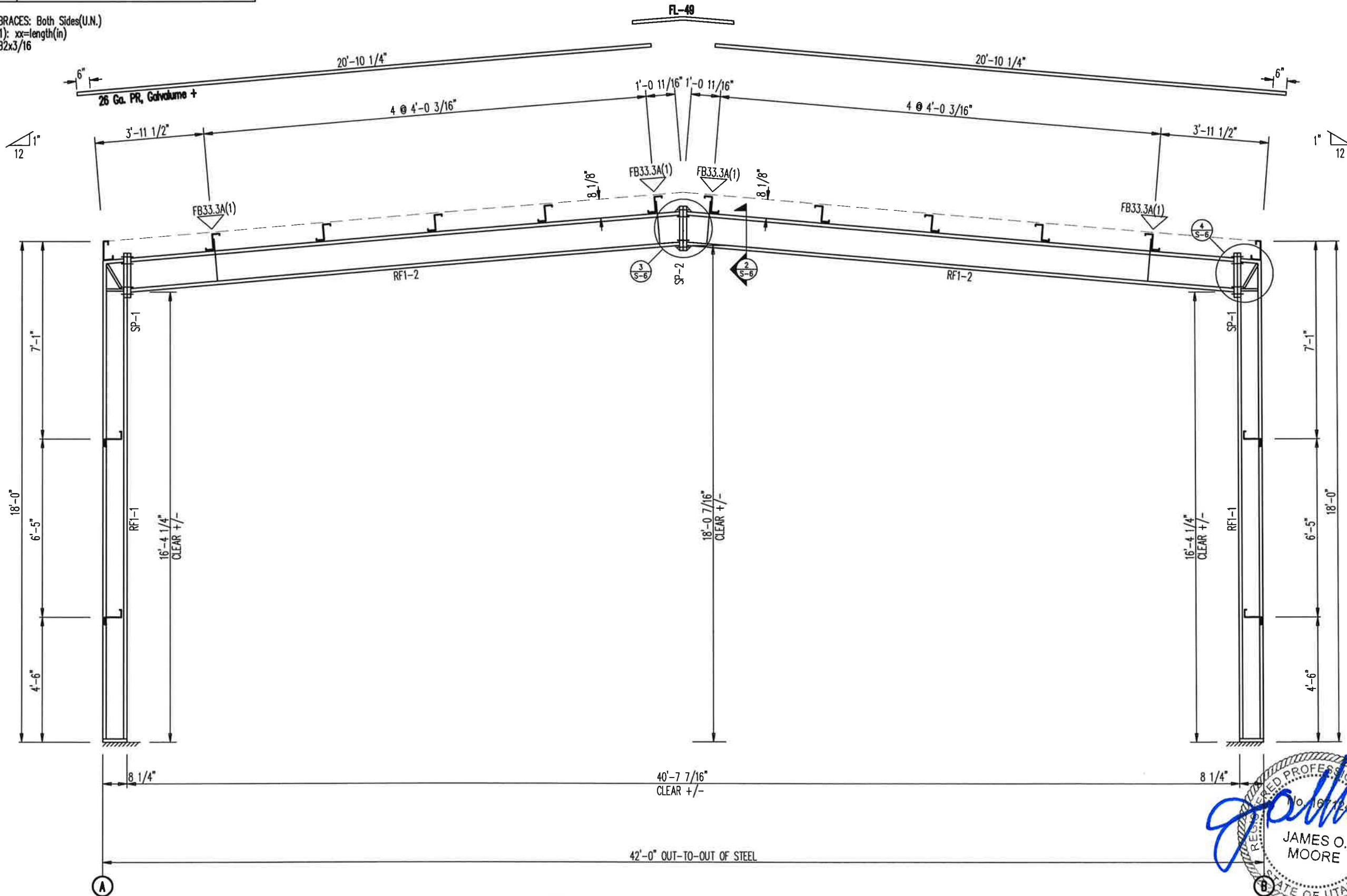
SHEET NUMBER:  
**S-3**

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
MVE JOB NUMBER:	170669

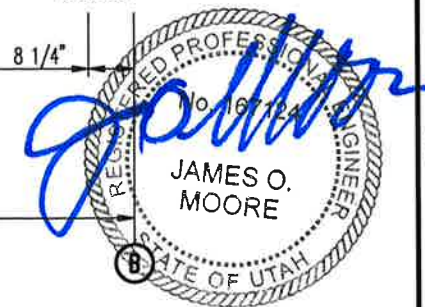
SPLICE BOLT TABLE						
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	0.625	2.50
SP-2	4	4	0	A325	0.625	1.75

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF1-1	W8X21	17'-3 7/8"	410
RF1-2	W12X19	20'-4 1/16"	427

FLANGE BRACES: Both Sides(U.N.)  
 FBxxA(1): xx=length(in)  
 A - FB2x3/16



RIGID FRAME ELEVATION: FRAME LINE 1 6



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SHEET TITLE: RIGID FRAME ELEVATION  
 JOB NAME: DESERT INDUSTRIES COVER  
 LOCATION: AMERICAN FORK, UTAH  
 CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	J.J. FOR PERMIT

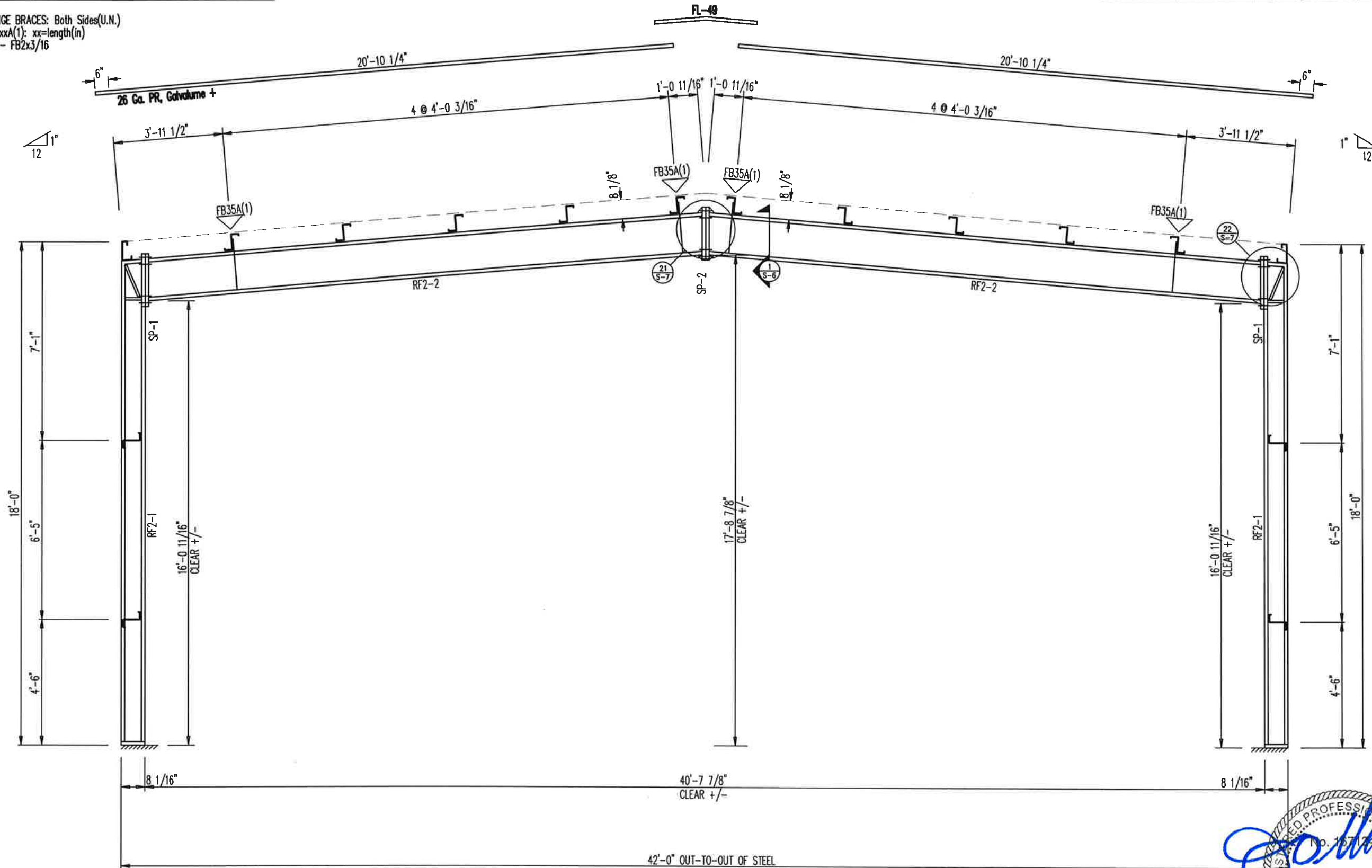
SHEET NUMBER:  
**S-4**

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
MVE JOB NUMBER:	170669

SPLICE BOLT TABLE						
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325	0.625	2.50
SP-2	4	4	0	A325	0.750	2.00

MEMBER SIZE TABLE			
MARK	MEMBER	LENGTH	WEIGHT
RF2-1	W8X28	17'-3 7/8"	544
RF2-2	W16X26	20'-4 5/16"	584

FLANGE BRACES: Both Sides(U.N.)  
 FBxxA(1): xx=length(in)  
 A - FB2x3/16



RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5

*[Signature]*  
 JAMES O. MOORE  
 ENGINEER  
 STATE OF UTAH  
 No. 157124

OCT 25 2017

**MOUNTAIN VIEW ENGINEERING, INC.**  
 Structural Engineering Consulting  
 345 North Main Street Suite A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

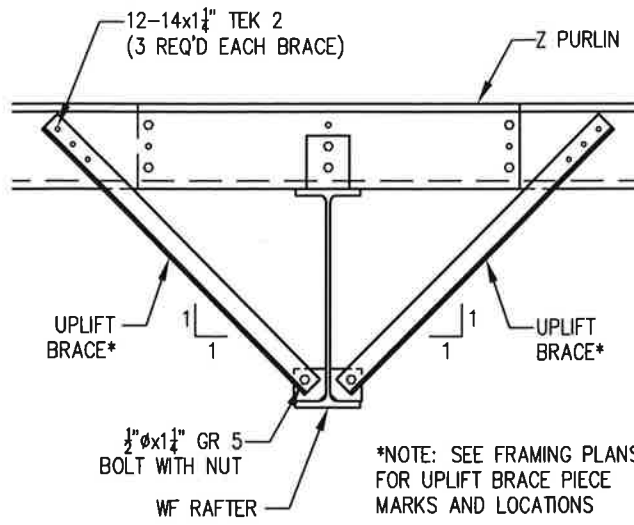
SHEET TITLE: RIGID FRAME ELEVATION  
 JOB NAME: DESERT INDUSTRIES COVER  
 LOCATION: AMERICAN FORK, UTAH  
 CONTRACTOR: PRECISION CANOPY

PLAN ISSUE DATES	
DATE	DESCRIPTION
10-23-17	J.J. FOR PERMIT

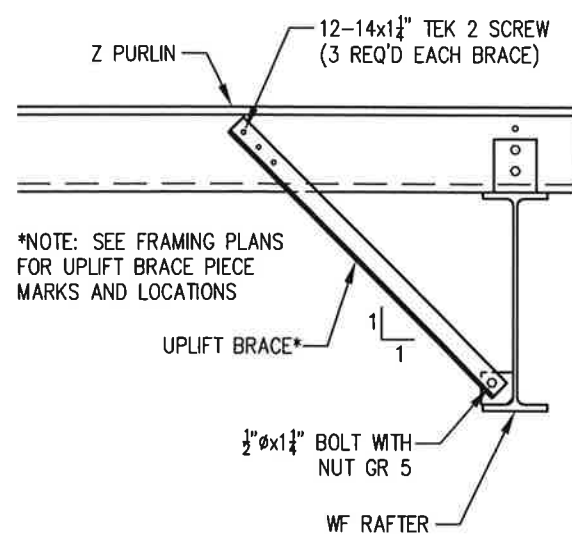
SHEET NUMBER:  
**S-5**

DRAWN BY:	J.J.
ENGINEER:	J.O. MOORE
MVE JOB NUMBER:	170669

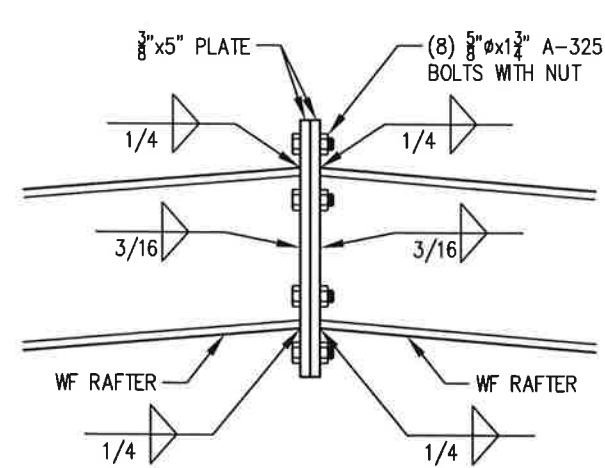




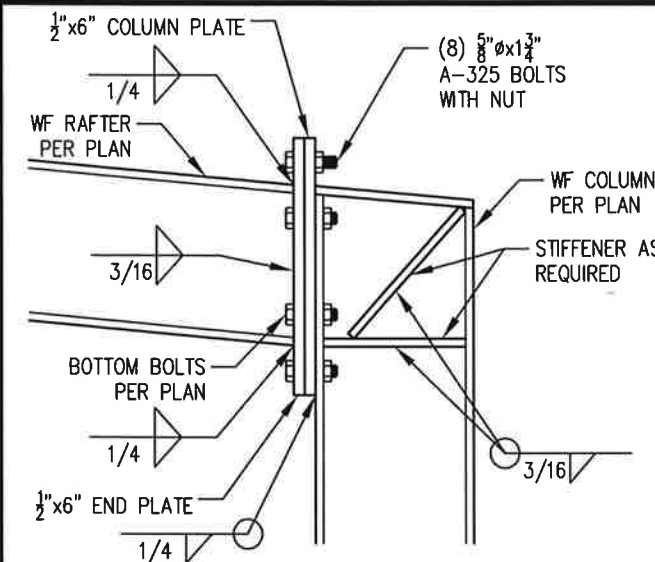
① UPLIFT BRACE TO Z PURLIN  
1"=1'-0"



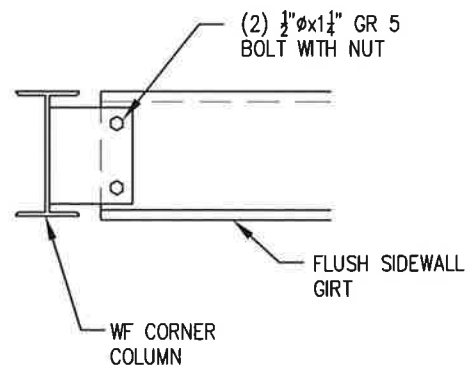
② UPLIFT BRACE TO Z PURLIN  
1"=1'-0"



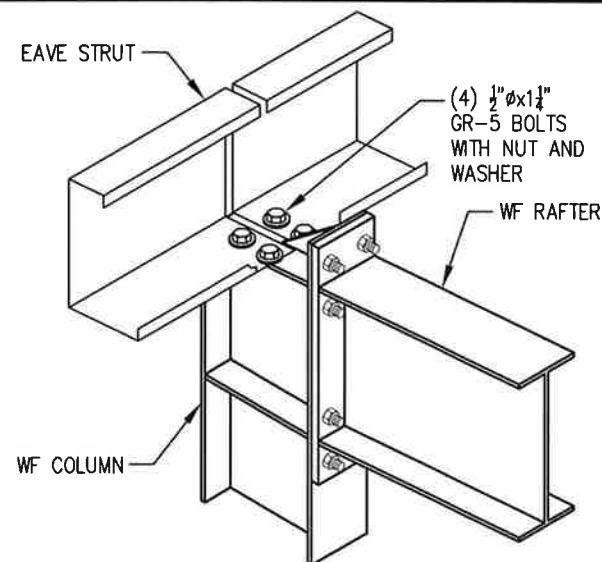
③ RIDGE CONNECTION DETAIL  
1"=1'-0" @ LINES 1 AND 6



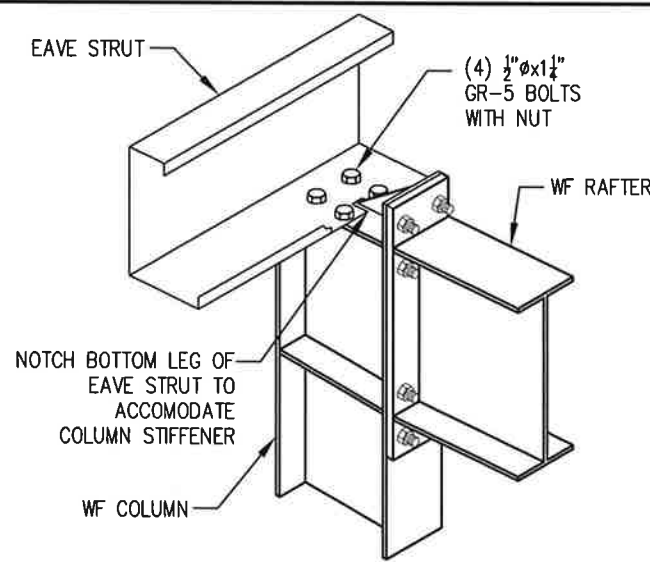
④ RAFTER TO COLUMN CONNECTION  
1"=1'-0" @ LINES 1 AND 6



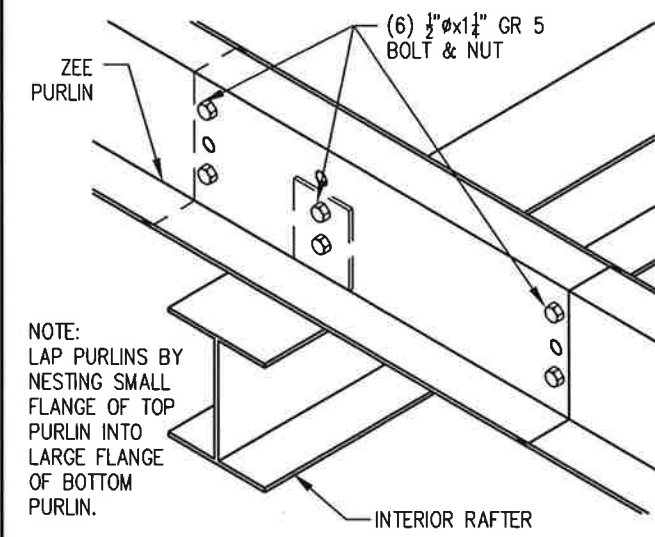
⑤ CORNER COLUMN DETAIL  
1"=1'-0"



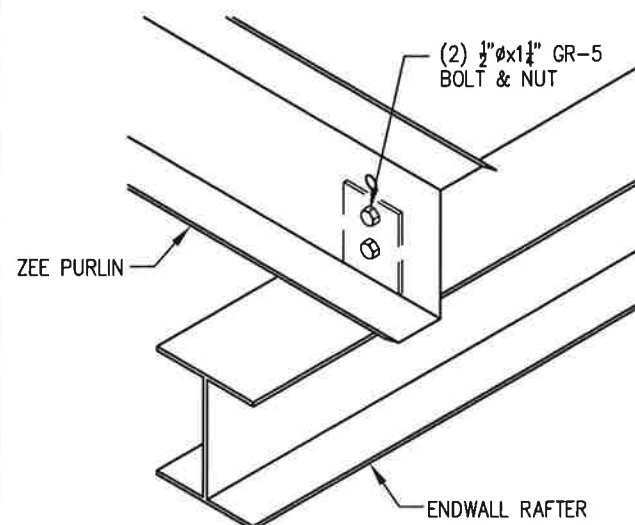
⑥ EAVE STRUT TO INTERIOR COLUMN  
1"=1'-0"



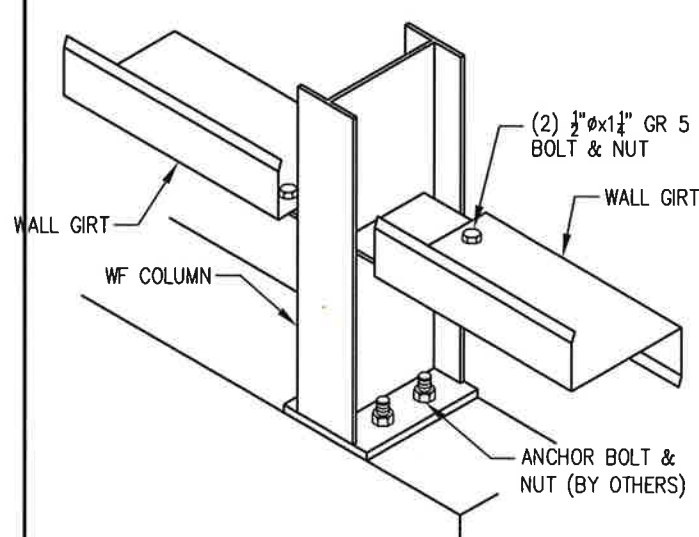
⑦ EAVE STRUT TO ENDWALL COLUMN  
1"=1'-0"



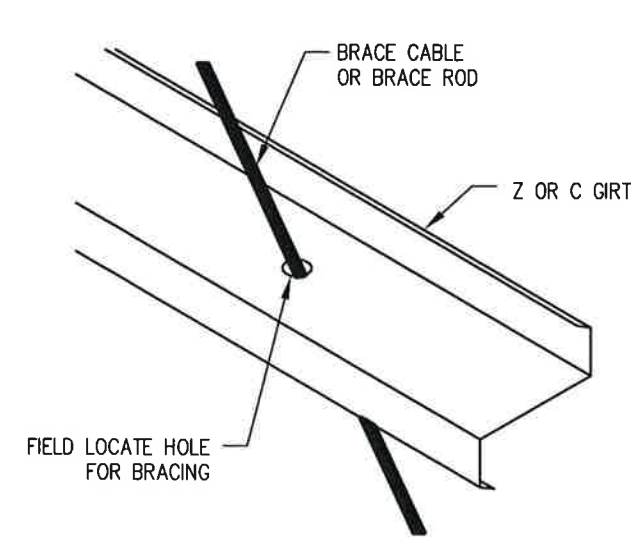
⑧ Z PURLIN TO RAFTER  
1"=1'-0"



⑨ Z PURLIN TO RAFTER  
1"=1'-0"



⑩ FLUSH GIRTS TO COLUMN  
1"=1'-0"



⑪ BRACING THROUGH GIRTS  
1"=1'-0"



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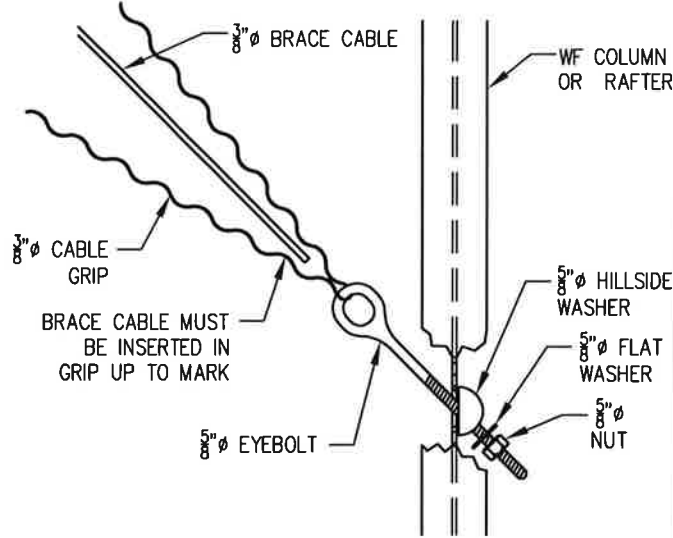
**MOUNTAIN VIEW ENGINEERING, INC.**  
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SHEET TITLE: ERECTION DETAILS  
JOB NAME: DESERET INDUSTRIES COVER  
LOCATION: AMERICAN FORK, UTAH  
CONTRACTOR: PRECISION CANOPY

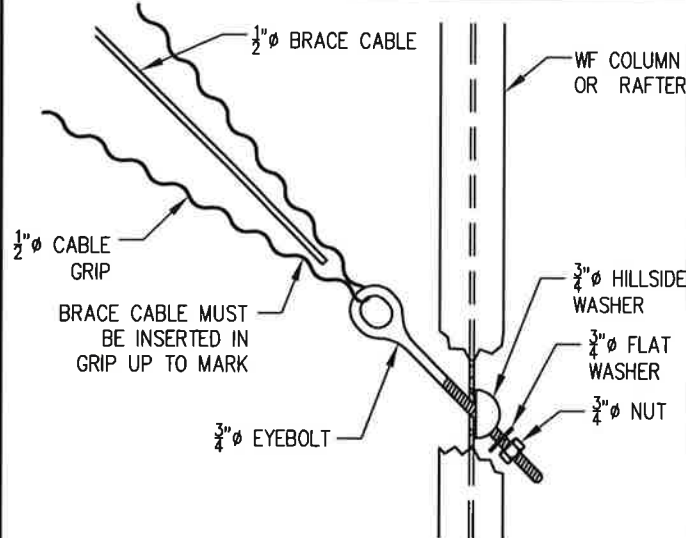
PLAN ISSUE DATES	DATE	BY	DESCRIPTION
	10-23-17	J.J.	FOR PERMIT

SHEET NUMBER:  
**S-6**

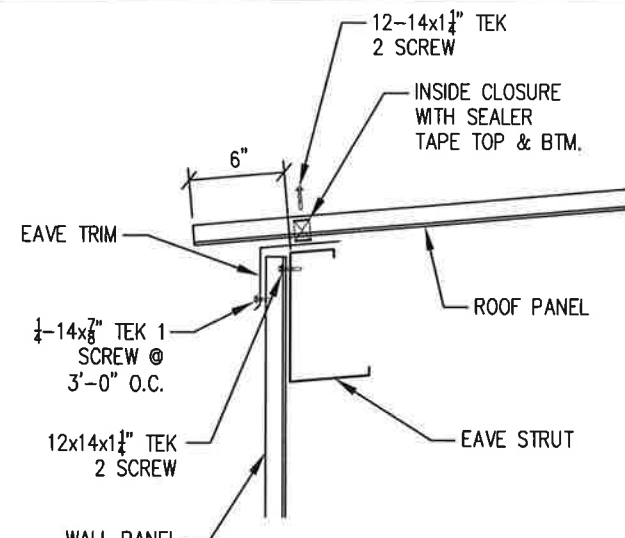
DRAWN BY: J.J.  
ENGINEER: J.O. MOORE  
MVE JOB NUMBER: 170669



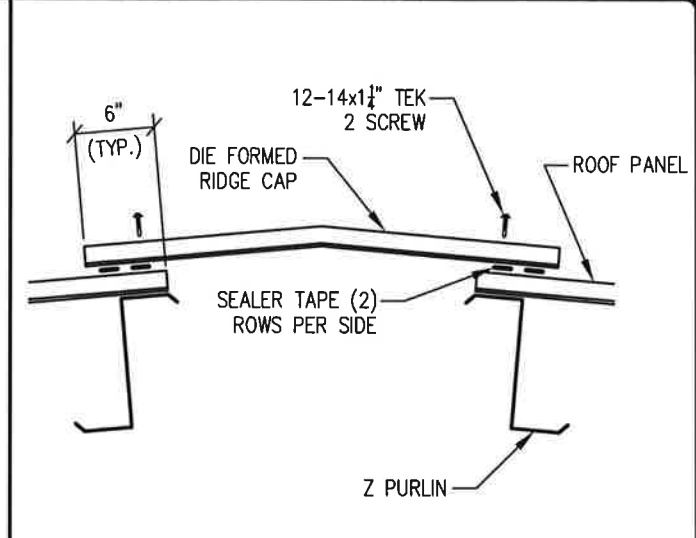
12  $\frac{3}{8}$ "  $\phi$  BRACE CABLE TO WF RAFTER  
1"=1'-0"



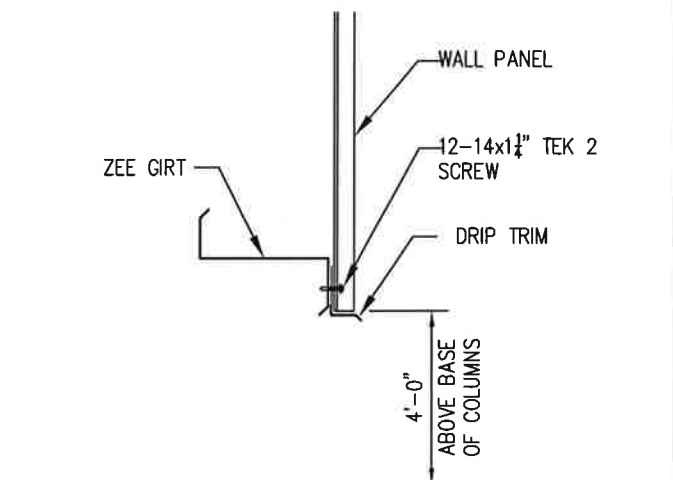
13  $\frac{1}{2}$ " BRACE CABLE TO WF COLUMN  
1"=1'-0"



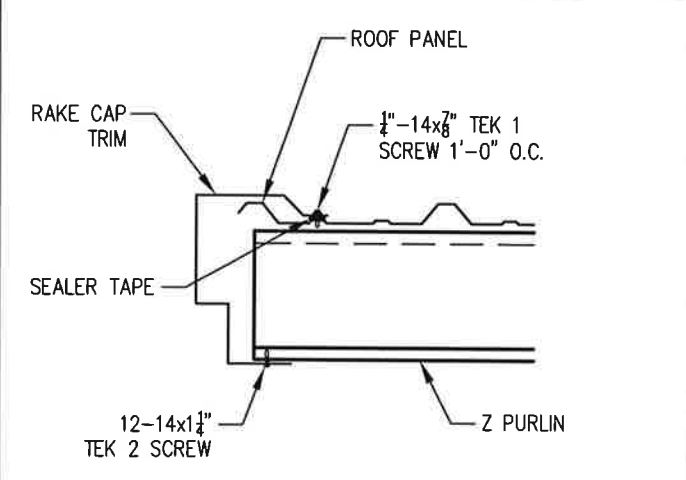
14 EAVE TRIM DETAIL  
1"=1'-0"



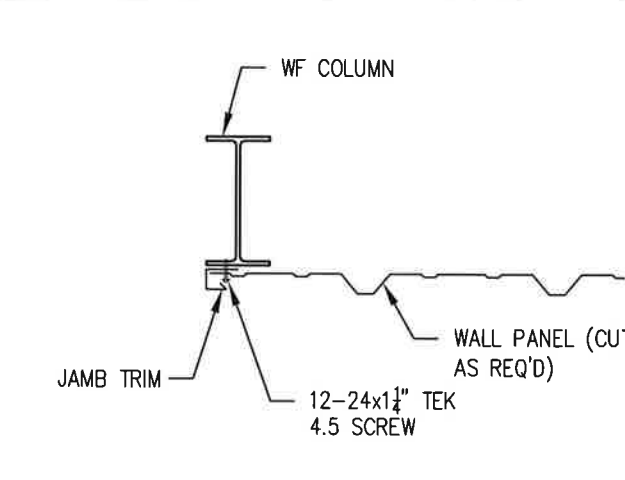
15 RIDGE CAP DETAIL  
1"=1'-0"



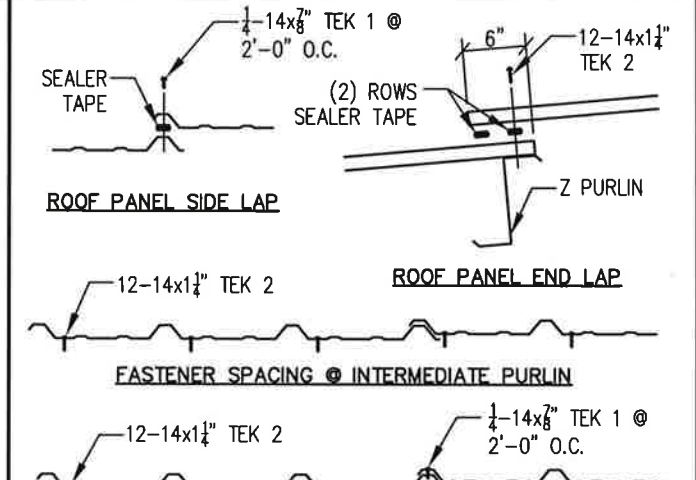
16 PANEL DRIP TRIM DETAIL  
1"=1'-0"



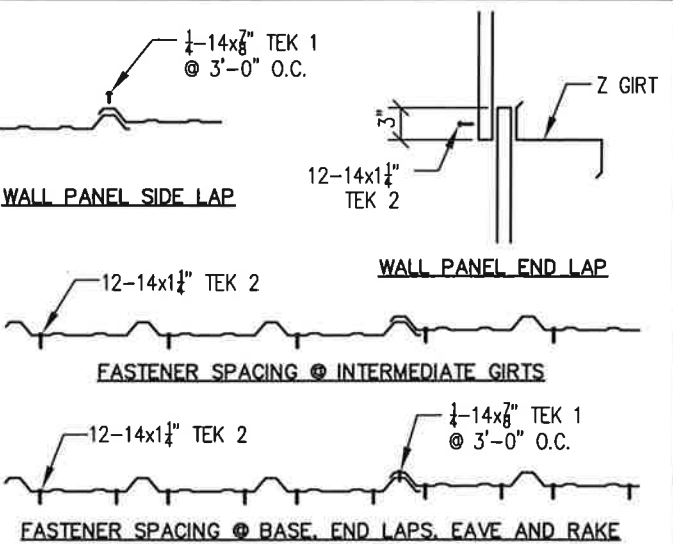
17 RAKE CAP TRIM DETAIL  
1"=1'-0"



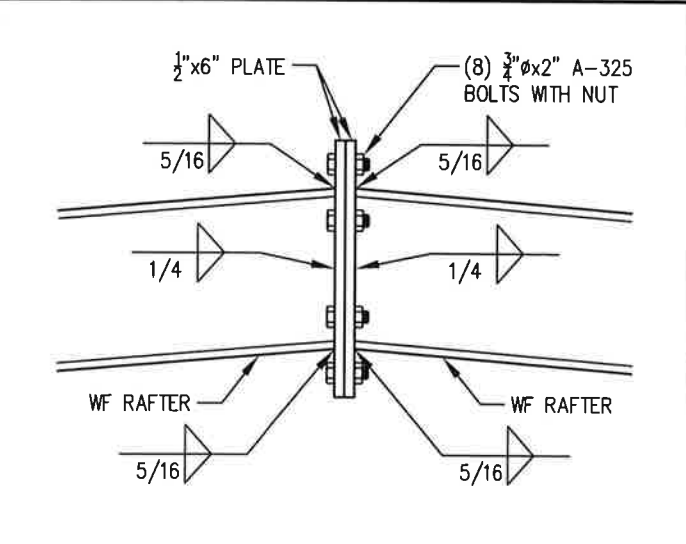
18 PANEL CAP TRIM DETAIL  
1"=1'-0"



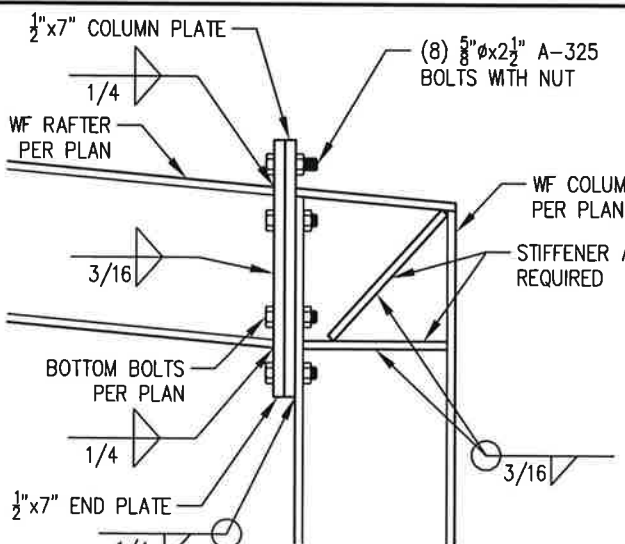
19 ROOF PANEL FASTENER SPACING  
N.T.S.



20 WALL PANEL FASTENER SPACING  
N.T.S.

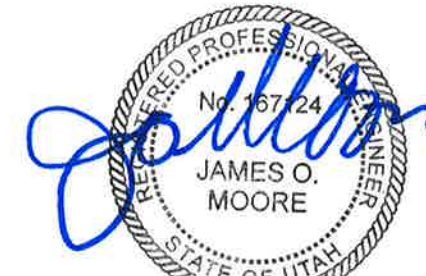


21 RIDGE CONNECTION DETAIL  
1"=1'-0" @ LINES 2 THRU 5



22 RAFTER TO COLUMN CONNECTION  
1"=1'-0" @ LINES 2 THRU 5

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SHEET NUMBER:  
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